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(12) United States Patent
White**(10) Patent No.: US 9,567,723 B2****(45) Date of Patent: *Feb. 14, 2017****(54) OPEN-END EXTENSIBLE SHELLS AND
RELATED METHODS FOR CONSTRUCTING
A SUPPORT PIER*****E02D 23/00*** (2013.01); ***E02D 27/18***
(2013.01); ***E02D 27/20*** (2013.01)**(71) Applicant: Geopier Foundation Company, Inc.,
Davidson, NC (US)****(58) Field of Classification Search**CPC ***E02D 3/08***; ***E02D 7/28***; ***E02D 23/00***;
E02D 27/18; ***E02D 27/20***; ***E02D 27/28***
See application file for complete search history.**(72) Inventor: David J. White, Boone, IA (US)****(56) References Cited****(73) Assignee: GEOPIER FOUNDATION
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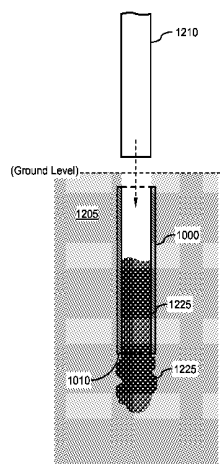
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JP 07-197442 A 8/1995**(22) Filed: Jul. 27, 2015***Primary Examiner* — Frederick L Lagman**(65) Prior Publication Data**

US 2015/0330051 A1 Nov. 19, 2015

(74) Attorney, Agent, or Firm — Nexsen Pruet, PLLC; E.
Eric Mills**Related U.S. Application Data****(57) ABSTRACT****(63)** Continuation-in-part of application No. 13/822,011,
filed as application No. PCT/US2011/051349 on Sep.
13, 2011, now Pat. No. 9,091,036, which is a
continuation-in-part of application No. 12/880,804,
filed on Sep. 13, 2010, now Pat. No. 8,221,033.Extensible shells and related methods for constructing a
support pier are disclosed. An extensible shell can define an
interior for holding granular construction material and define
a first opening at a first end for receiving the granular
construction material into the interior and a second opening
at a second end. The extensible shell can be flexible such that
the shell expands when granular construction material is
compacted in the interior of the shell. A method may include
positioning the extensible shell in the ground and filling at
least a portion of the interior of the shell with the granular
construction material. The granular construction material
may be compacted in the interior of the extensible shell to
form a support pier.**(51) Int. Cl.*****E02D 3/08*** (2006.01)
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E02D 27/18 (2006.01)
E02D 27/20 (2006.01)
E02D 23/00 (2006.01)**(52) U.S. Cl.**CPC . ***E02D 3/08*** (2013.01); ***E02D 7/28*** (2013.01);**24 Claims, 21 Drawing Sheets**

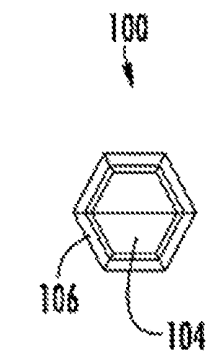
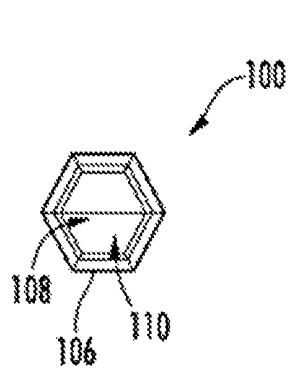
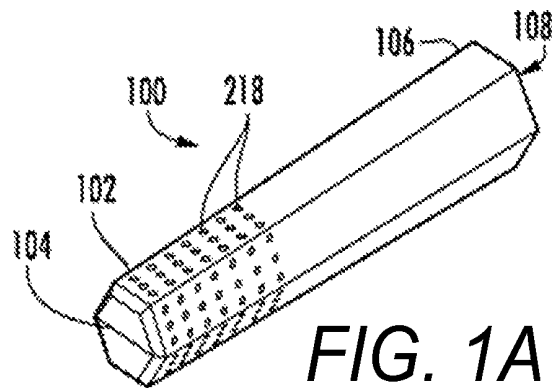
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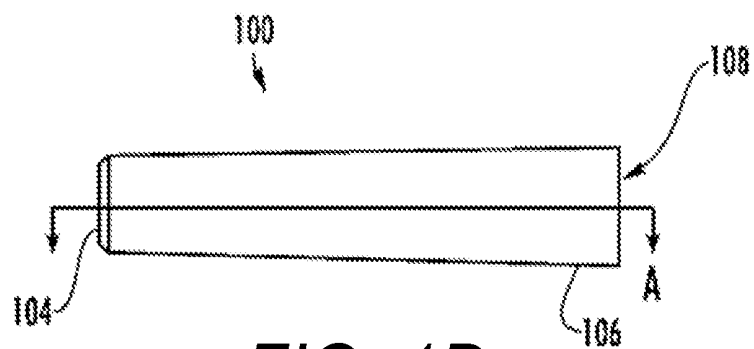


FIG. 1D



FIG. 1E

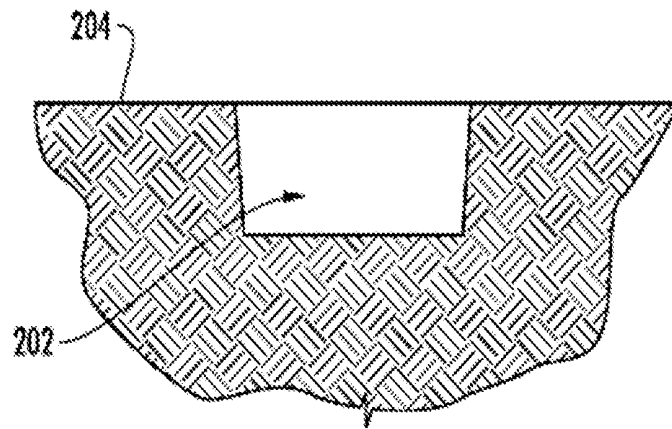


FIG. 2A

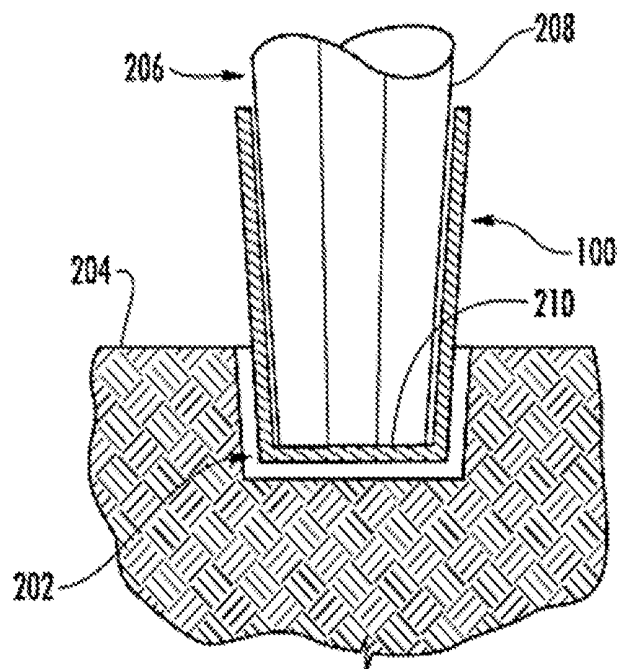


FIG. 2B

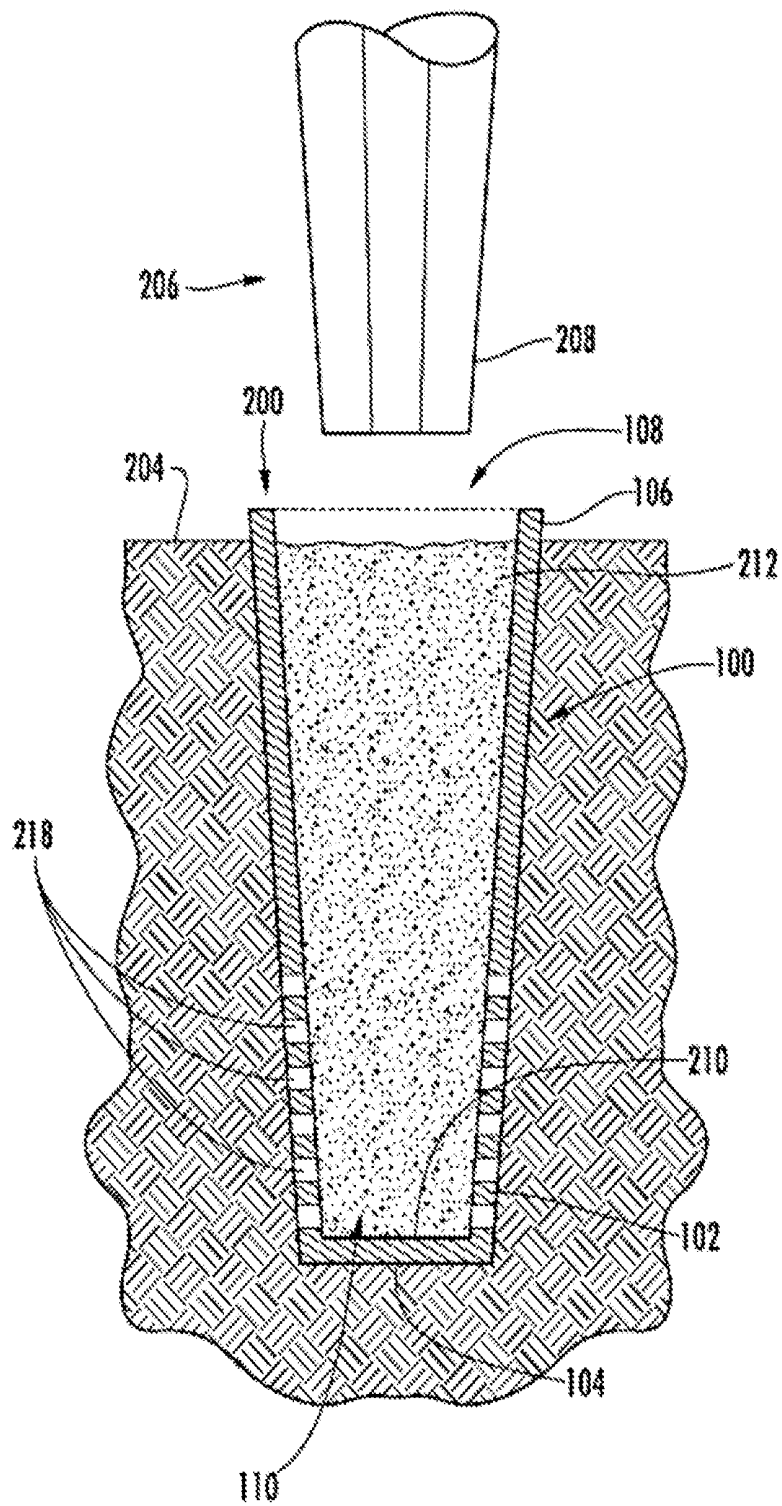
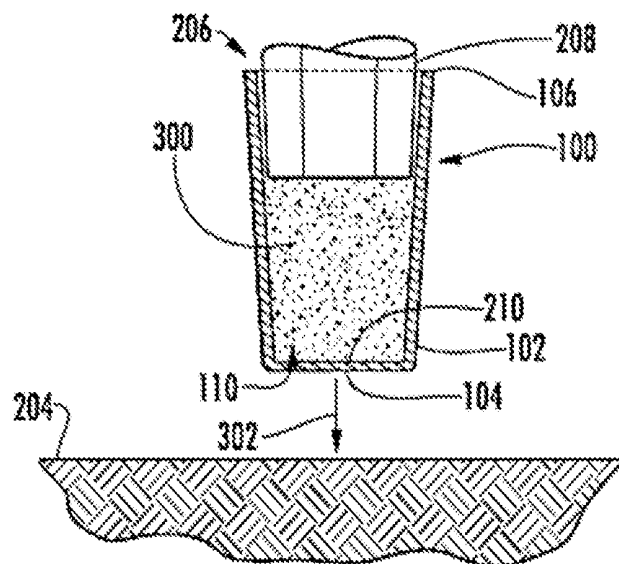
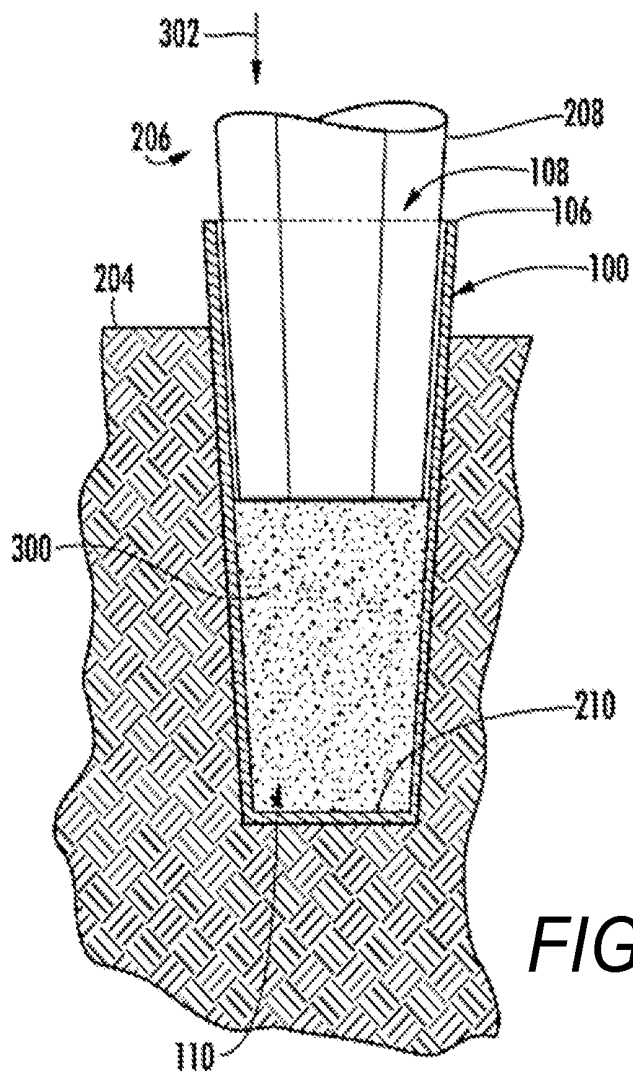
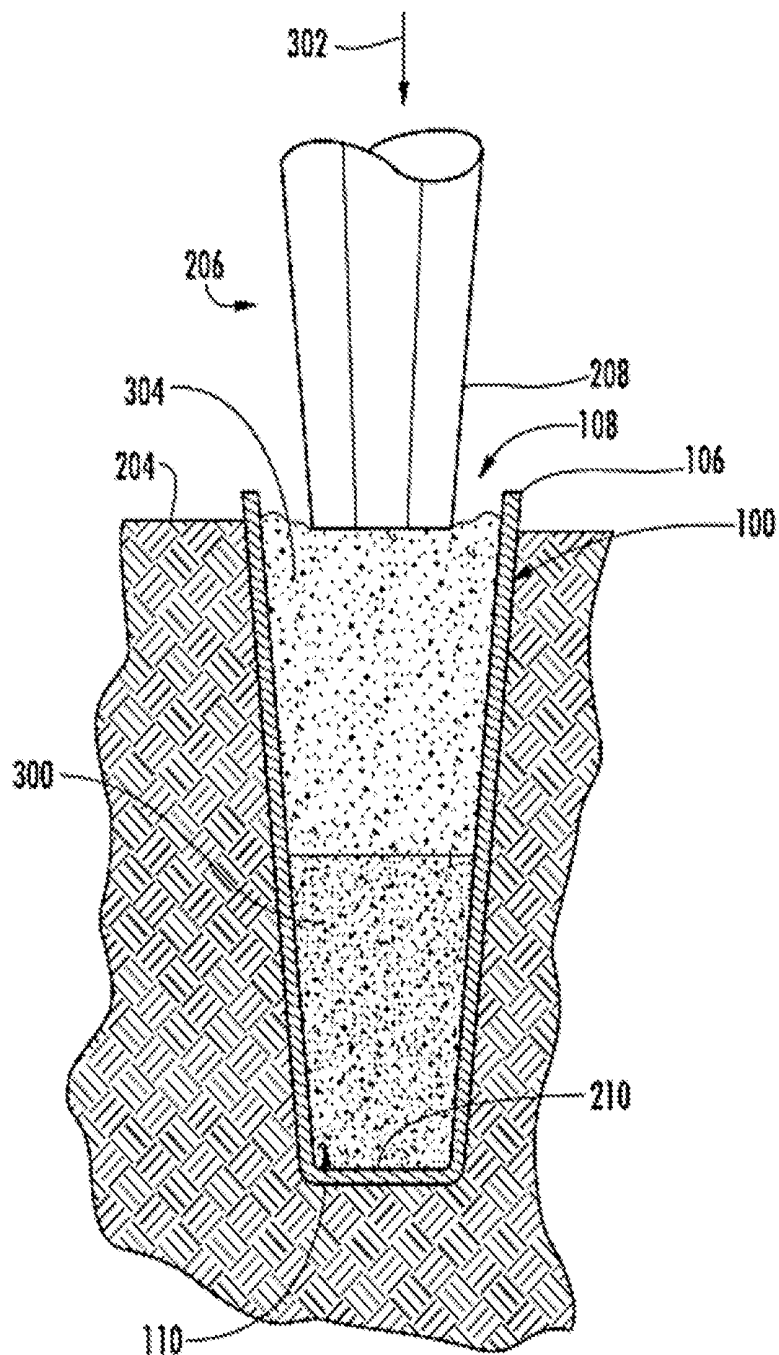
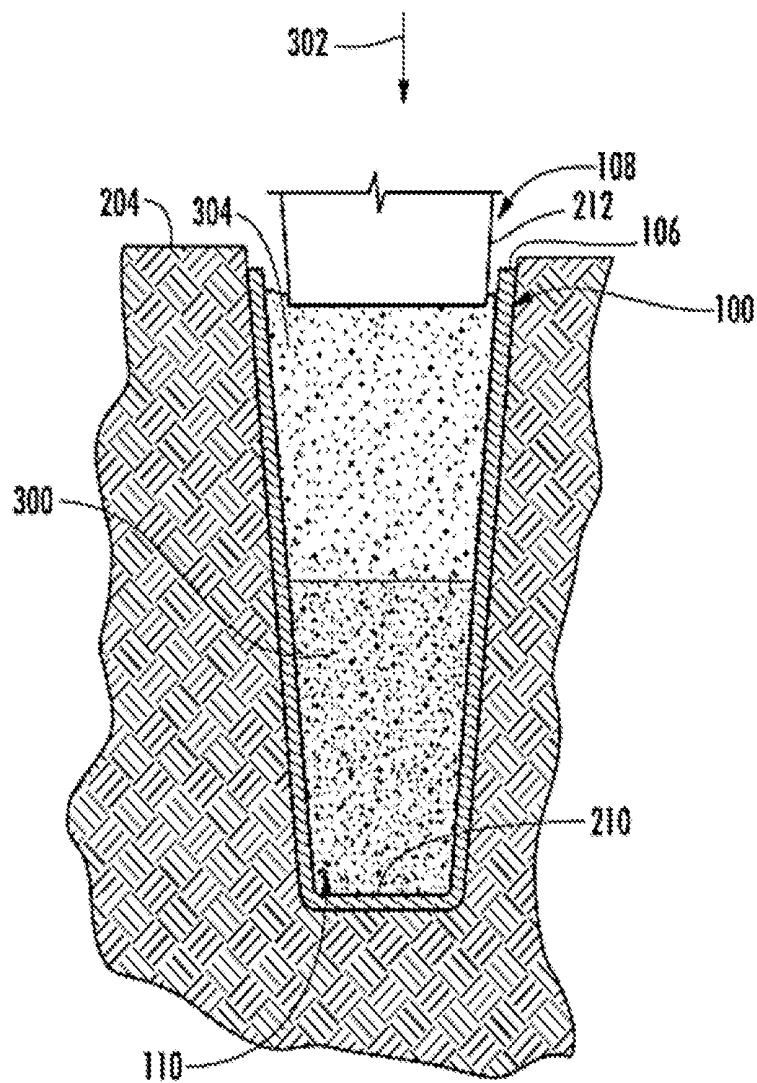


FIG. 2C

*FIG. 3A*

*FIG. 3B*

*FIG. 3C*

*FIG. 3D*

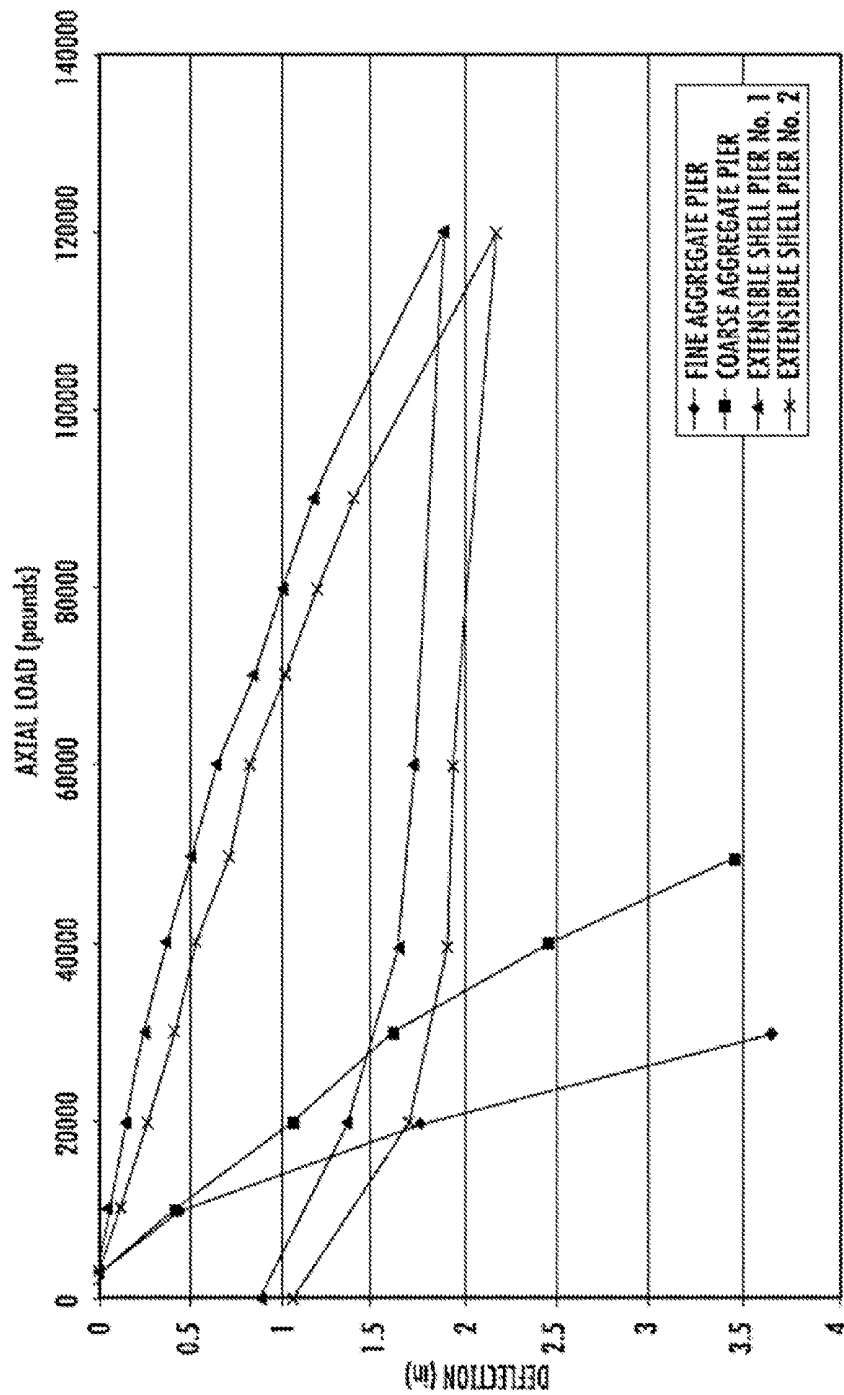


FIG. 4

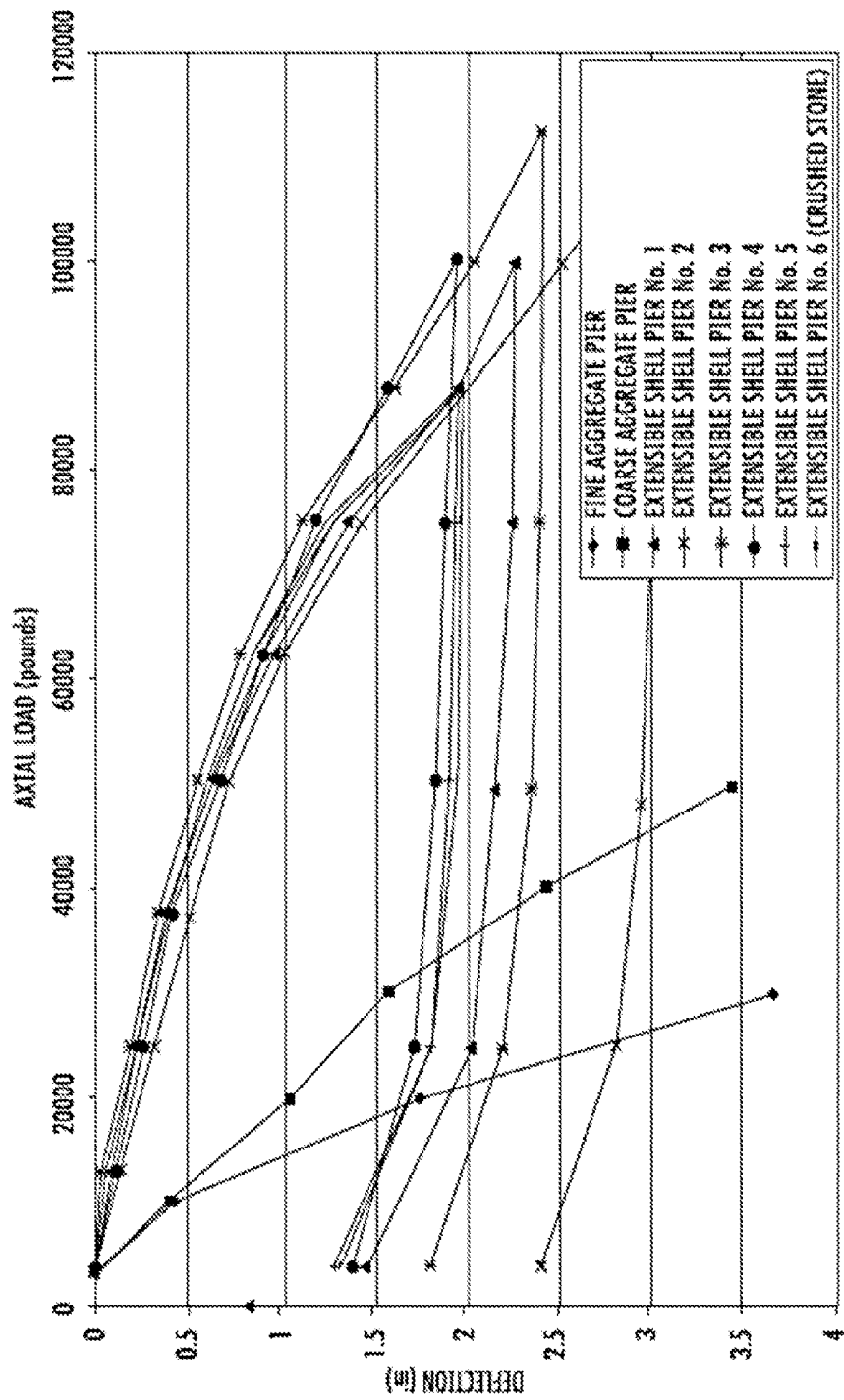


FIG. 5

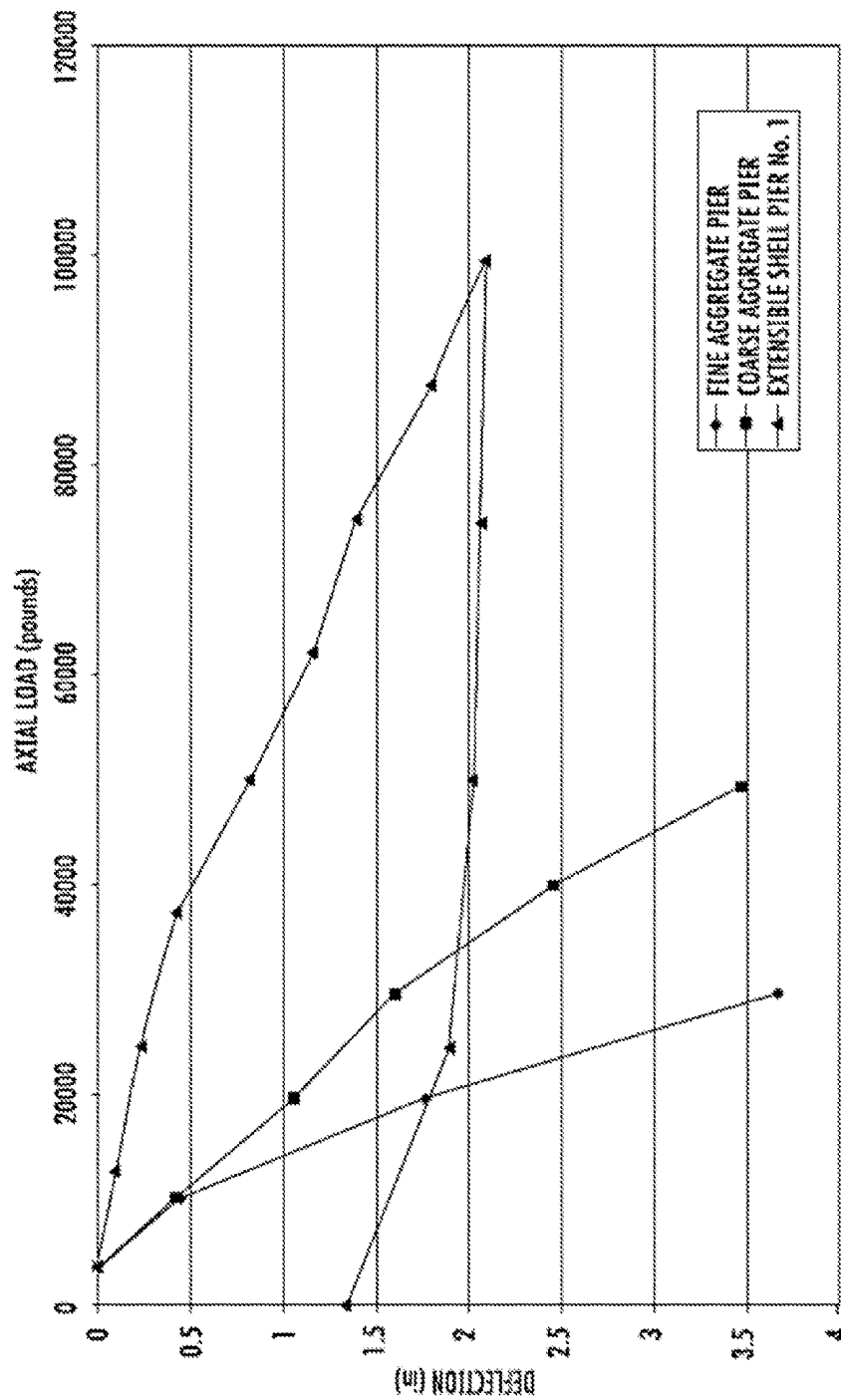


FIG. 6

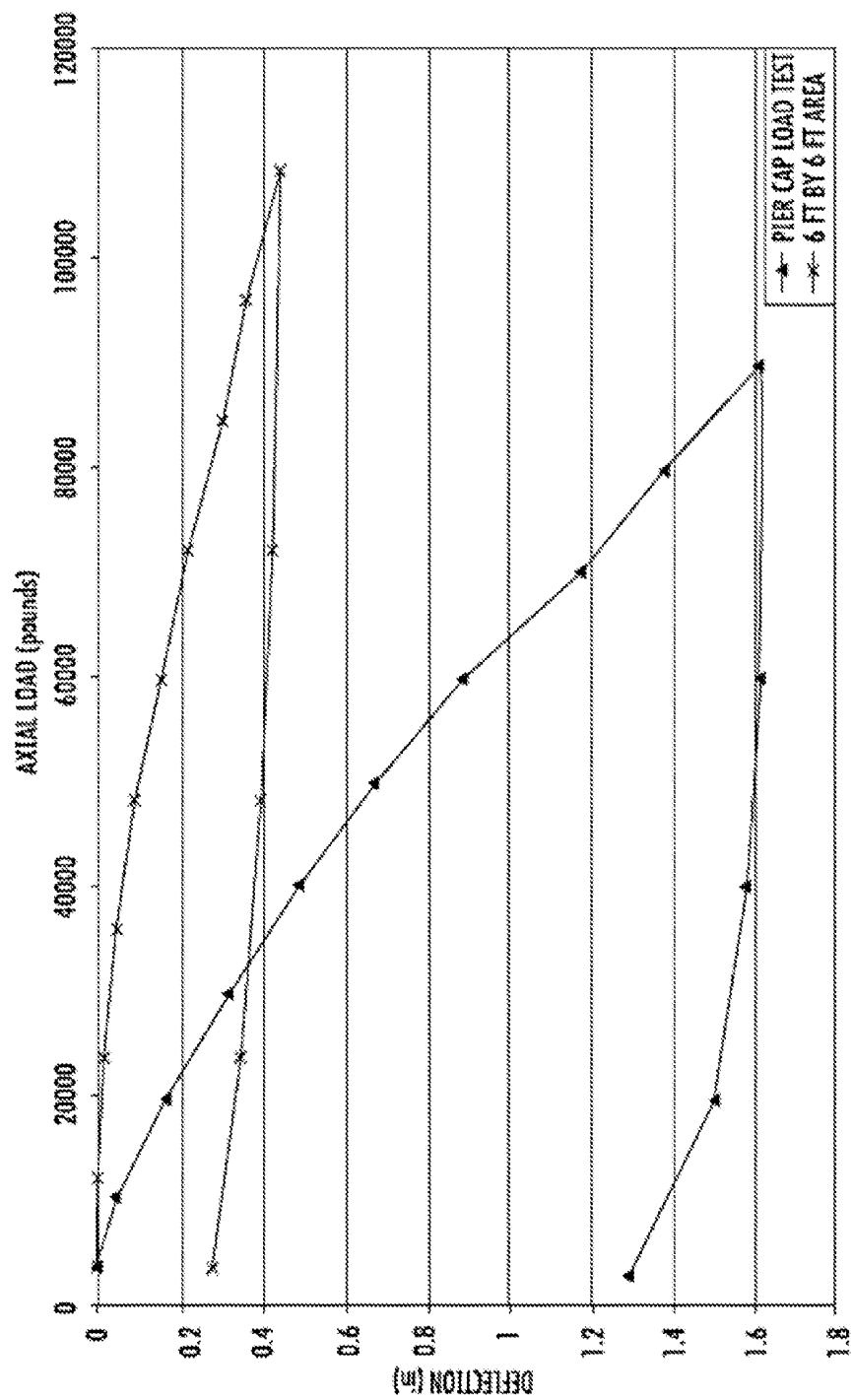
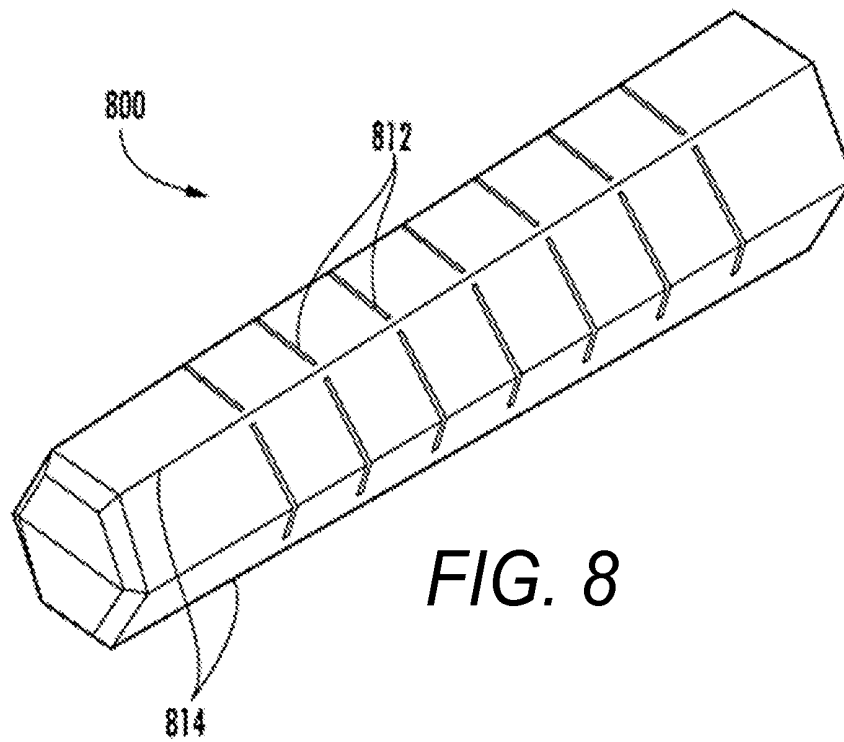


FIG. 7



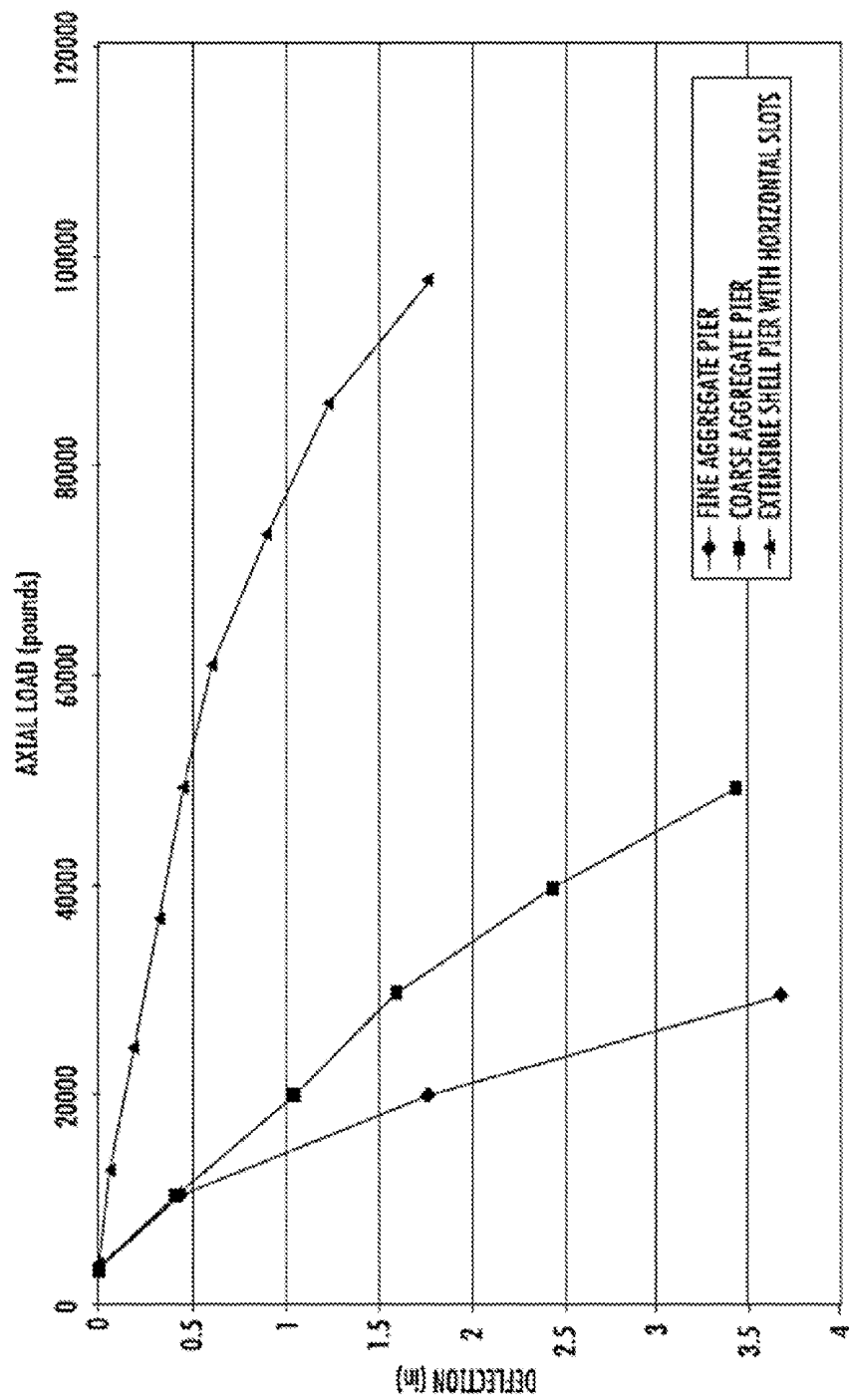


FIG. 9

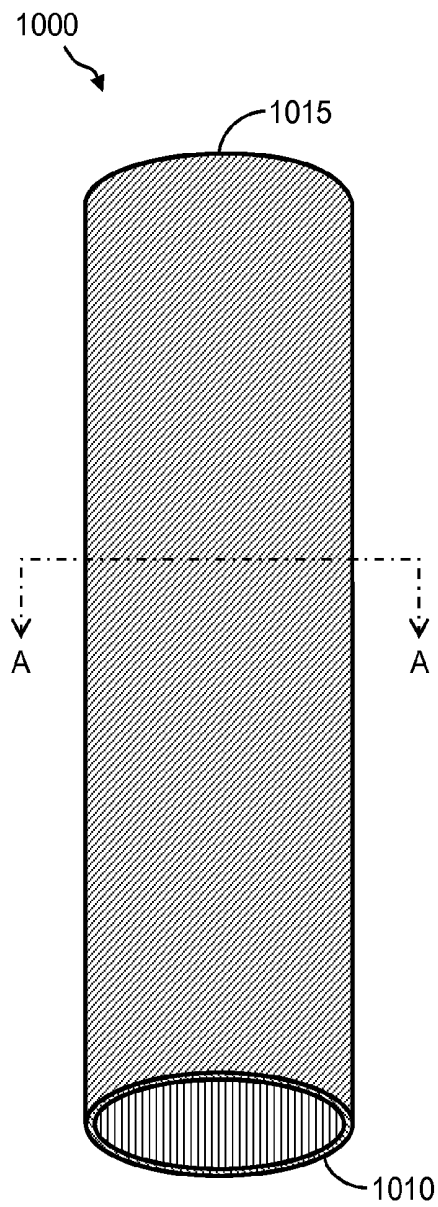


FIG. 10A

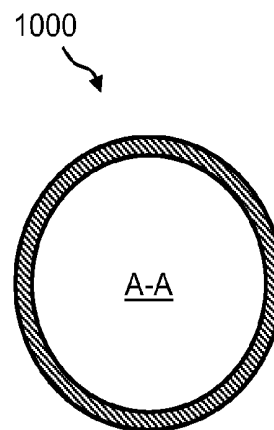


FIG. 10B

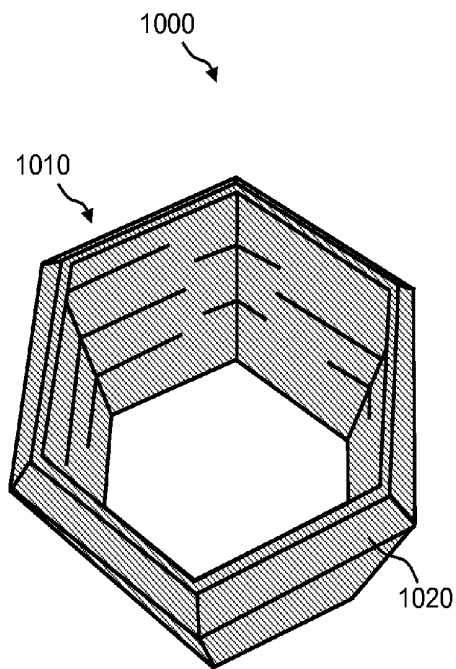


FIG. 11A

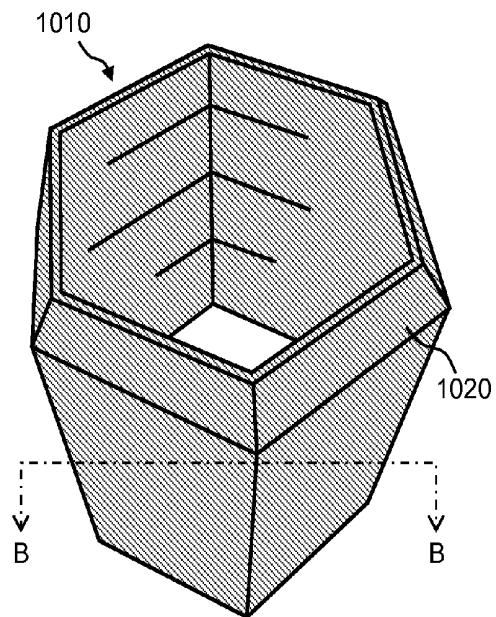


FIG. 11B

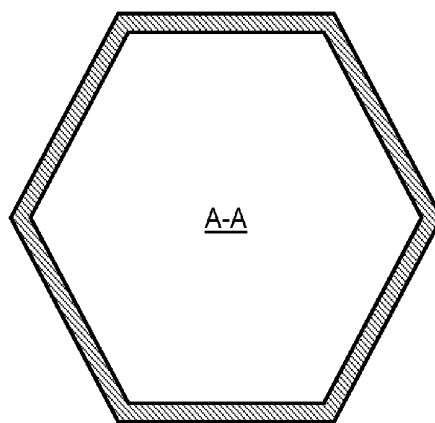


FIG. 11C

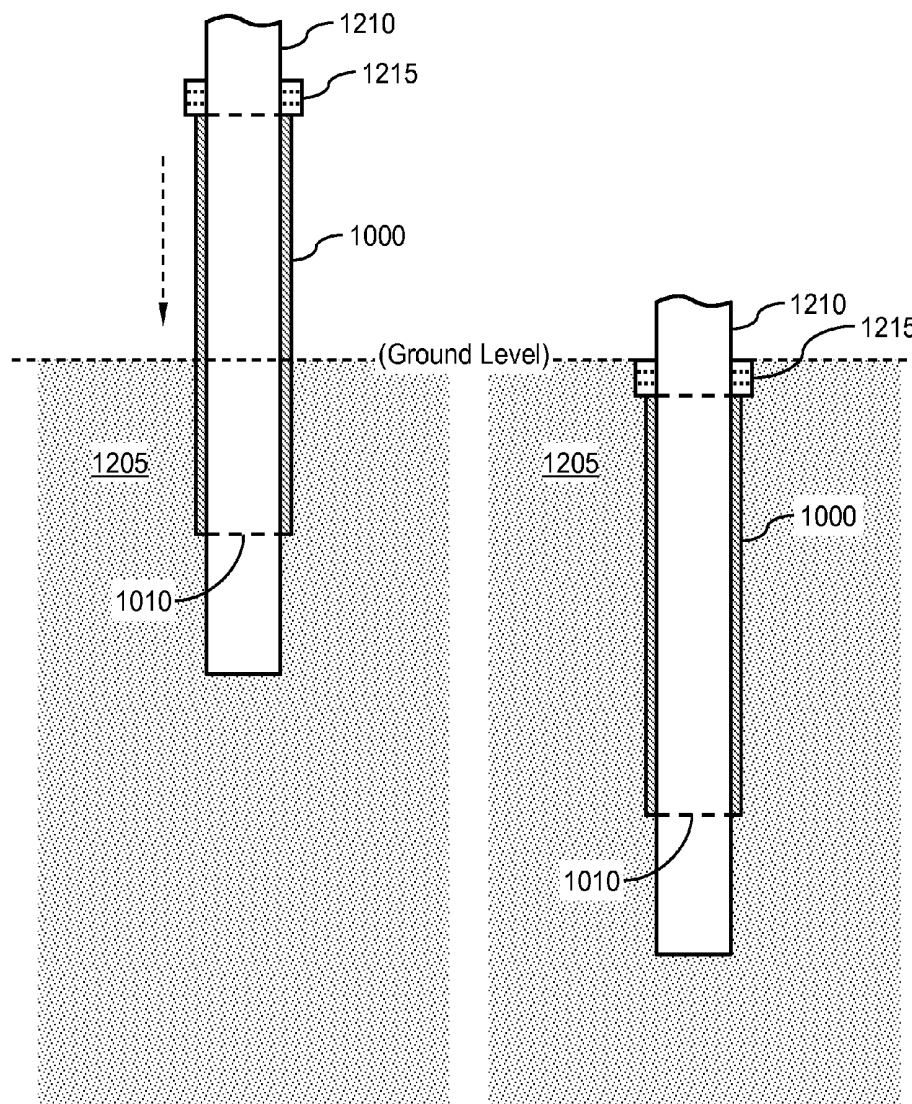
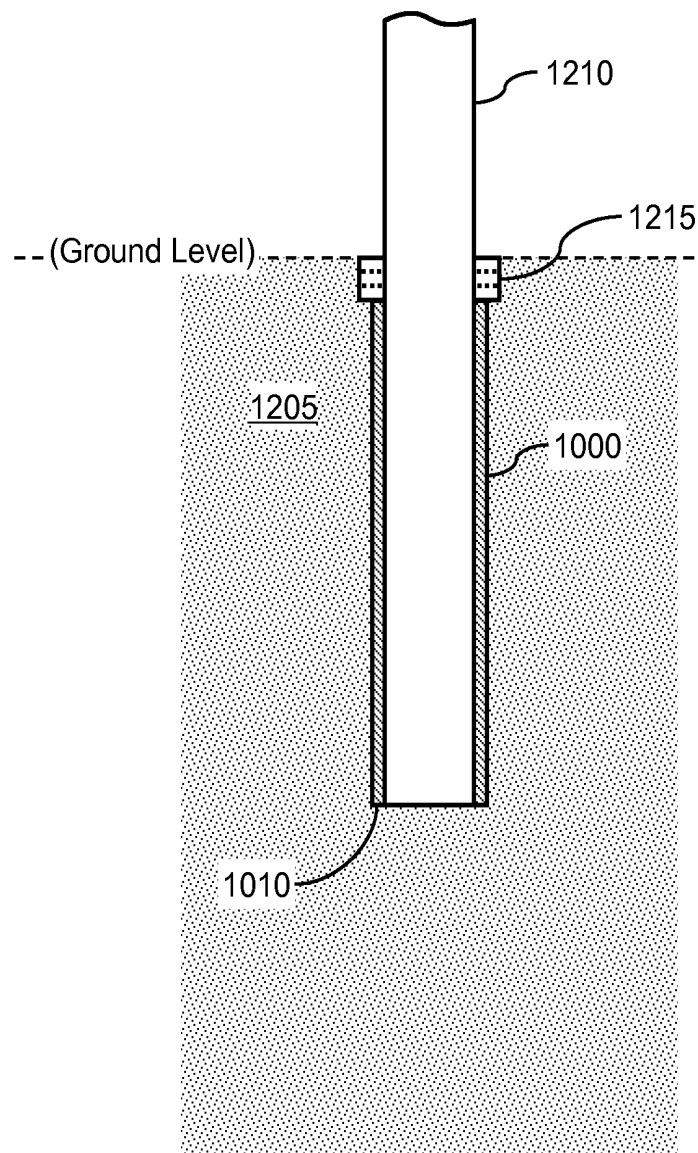
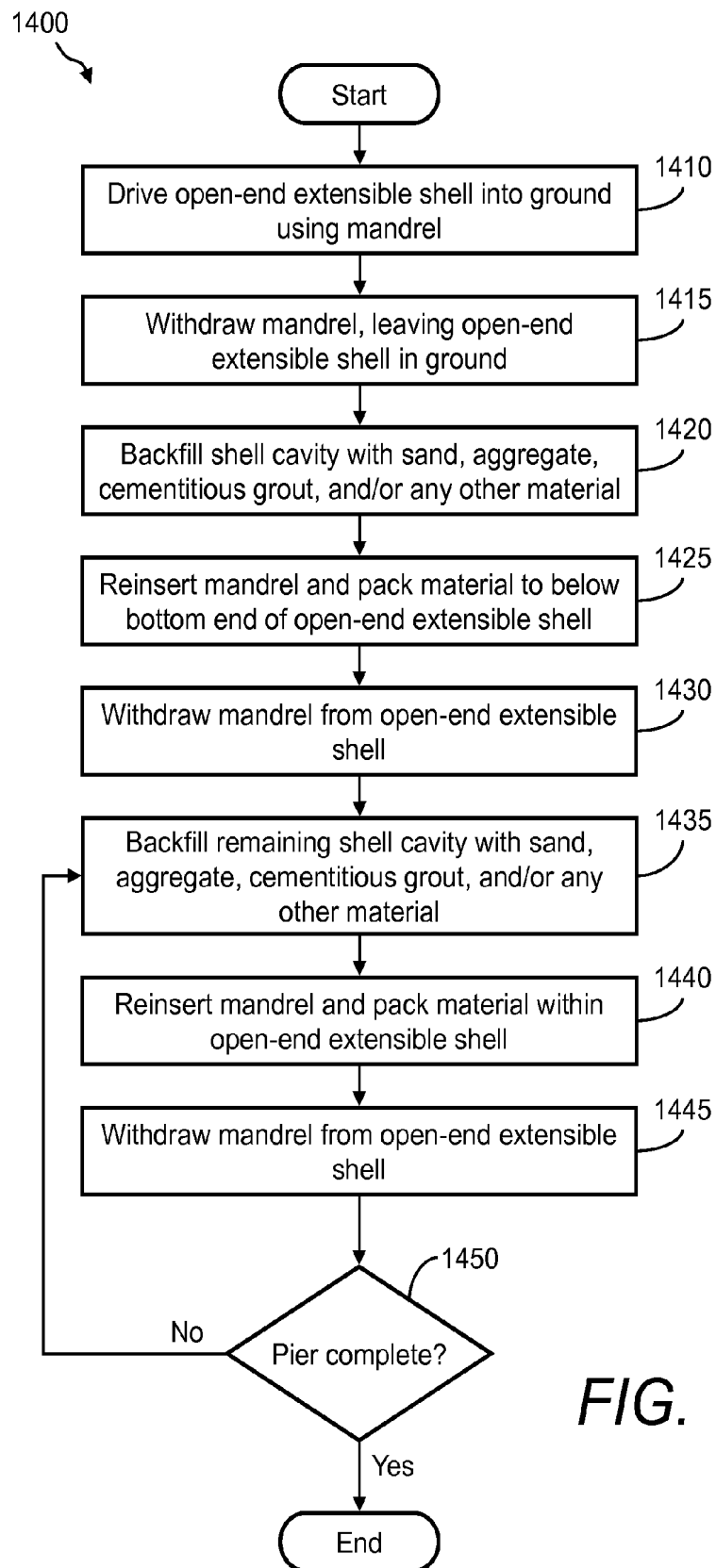


FIG. 12A

FIG. 12B

*FIG. 13*

**FIG. 14**

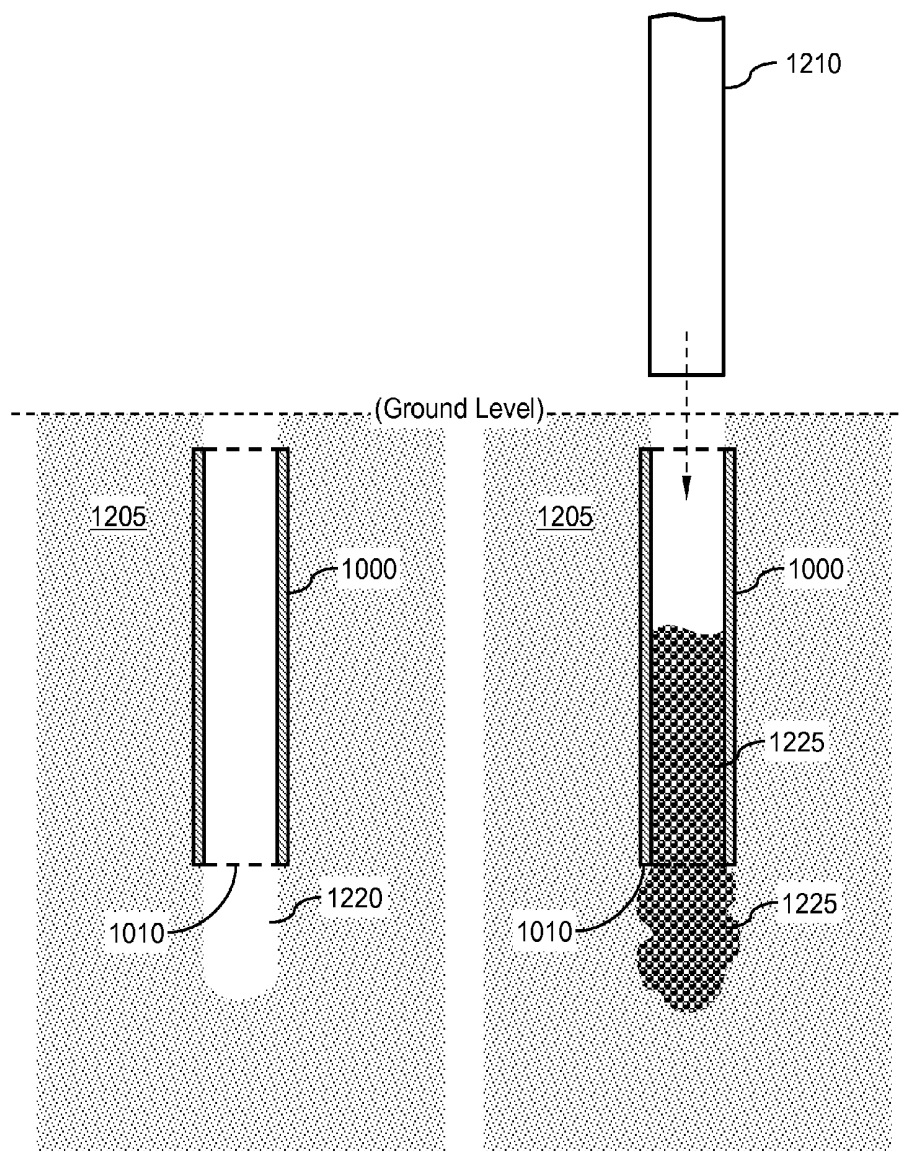


FIG. 15A

FIG. 15B

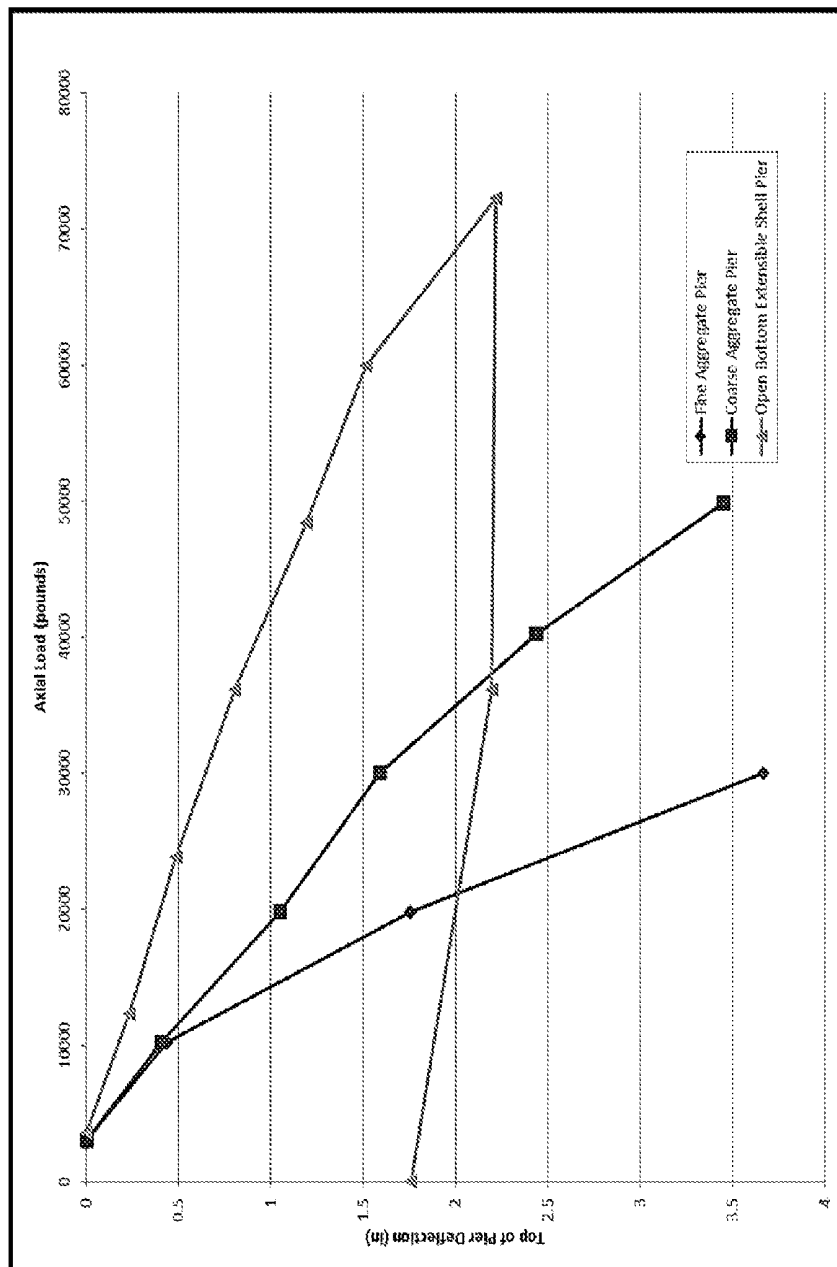


FIG. 16

OPEN-END EXTENSIBLE SHELLS AND RELATED METHODS FOR CONSTRUCTING A SUPPORT PIER

CROSS REFERENCE TO RELATED APPLICATION

This application is related to and claims the priority of U.S. patent application Ser. No. 13/822,011 filed Jun. 20, 2013 (issued as U.S. Pat. No. 9,091,036 on Jul. 28, 2015) which is a 35 U.S.C. §371 U.S. national phase entry of International Application No. PCT/US2011/051349 having an international filing date of Sep. 13, 2011, which is a continuation-in-part application of U.S. patent application Ser. No. 12/880,804 filed Sep. 13, 2010 (issued as U.S. Pat. No. 8,221,033 on Jul. 17, 2012), the disclosures of which are incorporated herein by reference in their entirety.

TECHNICAL FIELD

The present invention relates to ground or soil improvement apparatuses and methods. More specifically, the present invention relates to open-end extensible shells and related methods for constructing a support pier.

BACKGROUND ART

Buildings, walls, industrial facilities, and transportation-related structures typically consist of shallow foundations, such as spread footings, or deep foundations, such as driven pilings or drilled shafts. Shallow foundations are much less costly to construct than deep foundations. Thus, deep foundations are generally used only if shallow foundations cannot provide adequate bearing capacity to support building weight with tolerable settlements.

Recently, ground improvement techniques such as jet grouting, soil mixing, stone columns, and aggregate columns have been used to improve soil sufficiently to allow for the use of shallow foundations. Cement-based systems such as grouting or mixing methods can carry heavy loads but remain relatively costly. Stone columns and aggregate columns are generally more cost effective but can be limited by the load bearing capacity of the columns in soft clay soil.

Additionally, it is known in the art to use metal shells for the driving and forming of concrete piles. One set of examples includes U.S. Pat. Nos. 3,316,722 and 3,327,483 to Gibbons, which disclose the driving of a tapered, tubular metal shell into the ground and subsequent filling of the shell with concrete in order to form a pile. Another example is U.S. Pat. No. 3,027,724 to Smith which discloses the installation of shells in the earth for subsequent filling with concrete for the forming of a concrete pile. A disadvantage of these prior art shells is that their sole purpose is for providing a temporary form for the insertion of cementitious material for the forming of a hardened pile for structural load support. The prior art shells are not extensible and thus do not exhibit properties that allow them to engage the surrounding soil through lateral deformations. Further, because they relate to the use of ferrous materials, which are subject to corrosion, their function is complete once the concrete infill hardens. Thus, the prior art shells are not suitable for containing less expensive granular infill materials such as sand or aggregate, because the prior art shells cannot laterally contain the inserted materials during the life of the pier. The prior art shells are also not permeable and are thus ill-suited to drain cohesive soils.

Accordingly, it is desirable to provide improved techniques for constructing a shallow support pier in soil or the ground using extensible shells formed of relatively permanent material of a substantially non-corrosive or non-degradable nature for the containment of compacted aggregate therein.

BRIEF DESCRIPTION OF THE INVENTION

Extensible shells and related methods for constructing a support pier in ground are disclosed. An extensible shell may define an interior for holding granular construction material and may define an opening for receiving the granular construction material into the interior. The shell may be flexible such that the shell expands laterally outward when granular construction material is compacted in the interior of the shell.

According to one aspect, the shell may include a first end that defines the opening. The shell may be shaped to taper downward from the first end to an opposing second end of the shell.

According to another aspect, the second end of the shell may define a substantially flat, blunt surface.

According to yet another aspect, a cross-section of the shell may form one of a substantially hexagonal shape and a substantially octagonal shape along a length of the shell extending between the first and second ends.

According to a further aspect, a cross-section of the first end of the shell is sized larger than a cross-section of the second end.

According to a still further aspect, the shell is comprised of plastic.

According to another aspect, the shell may define a plurality of apertures extending between an interior of the shell to an exterior of the shell.

According to yet another aspect, the shell may be either substantially cylindrical in shape or substantially conical in shape.

According to an additional aspect, a method may include positioning the shell in the ground and filling at least a portion of the interior of the shell with the granular construction material. The granular construction material may be compacted in the interior of the shell to form a pier.

According to another aspect, a method may include forming a cavity in the ground. The cavity may be partially backfilled with aggregate construction material. Next, the shell may be positioned with the cavity and at least a portion of the interior of the shell filled with granular construction material. The granular construction material may then be compacted in the interior of the shell to form a pier. The compaction may be performed with a primary mandrel. Additional compacting may be performed with a second mandrel that has a larger cross-sectional area than the primary mandrel.

According to a further aspect, the extensible shell may comprise a plurality of slots extending between an interior of the shell to an exterior of the shell, the slots being generally transverse to a centerline along the length of the shell. The slots may be discontinuous around a circumference of the shell thereby maintaining portions of continuous material connectivity along the length of the shell. The slots may have a width in the range of ¼ inch (6.35 mm) to ¾ inch (9.53 mm) and may be spaced at a distance of 6 inches (152 mm) from one another.

According to a still further aspect, the disclosure is directed to an extensible shell for constructing a support pier in ground, the extensible shell defining an interior for

holding granular construction material and said extensible shell defining a first end having a first opening for receiving granular construction material into the interior and a second end having a second opening, wherein the shell is flexible such that the shell expands laterally outward when granular construction material is compacted in the interior of the shell.

In another aspect, the first end defines the first opening with the shell shaped to taper from the first end to opposing second end of the shell, with the second end comprising a second opening.

In yet another aspect, a method for constructing a support pier in ground is disclosed, the method comprising: positioning an extensible shell into ground, the shell defining an interior for holding granular construction material and defining a first opening at a first end for receiving granular construction material into the interior and a second opening at a second end, wherein the shell is flexible such that the shell expands laterally outward when granular construction material is compacted in the interior of the shell; filling at least a portion of the interior of the shell with granular construction material; and compacting the granular construction material in the interior of the shell to form a support pier.

In a further aspect, the disclosure is directed to a method for constructing a support pier in ground, with the method comprising: forming a cavity in the ground; partially back-filling the cavity with an aggregate construction material; positioning an extensible shell into the cavity, with the shell having a first end with a first opening and a second end having a second opening, with the shell defining an interior for holding granular construction material and defining an opening for receiving the granular construction material into the interior, wherein the shell is flexible such that the shell expands when granular construction material is compacted in the interior of the shell; filling at least a portion of the interior of the shell with the granular construction material; and compacting the granular construction material in the interior of the shell to form a support pier.

This brief description is provided to introduce a selection of concepts in a simplified form that are further described below in the detailed description of the invention. This brief description of the invention is not intended to identify key features or essential features of the claimed subject matter, nor is it intended to be used to limit the scope of the claimed subject matter. Further, the claimed subject matter is not limited to implementations that solve any or all disadvantages noted in any part of this disclosure.

BRIEF DESCRIPTION OF THE DRAWINGS

FIG. 1A, FIG. 1B, FIG. 1C, FIG. 1D, and FIG. 1E illustrate different views of an extensible shell in accordance with embodiments of the present invention;

FIG. 2A, FIG. 2B, and FIG. 2C illustrate steps in an exemplary method of constructing a pier in ground using an extensible shell in accordance with an embodiment of the present invention;

FIG. 3A, FIG. 3B, FIG. 3C, and FIG. 3D illustrate steps in another exemplary method of constructing a support pier in ground using an extensible shell in accordance with embodiments of the present invention;

FIG. 4, FIG. 5, FIG. 6, and FIG. 7 are graphs showing results of load tests of support piers constructed using an extensible shell in accordance with embodiments of the present invention;

FIG. 8 illustrates a perspective view of another embodiment of the present invention pertaining to a slotted shell;

FIG. 9 is a graph showing results of load tests of a support pier constructed using an embodiment as shown in FIG. 8;

FIG. 10A and FIG. 10B illustrate a perspective view and a cross-sectional view of an example of an open-end extensible shell in accordance with embodiments of the present invention;

FIG. 11A, FIG. 11B, and FIG. 11C illustrate perspective views and a cross-sectional view of another example of an open-end extensible shell in accordance with embodiments of the present invention;

FIG. 12A and FIG. 12B show an example of a process of installing the open-end extensible shell into the ground;

FIG. 13 shows another example of installing the open-end extensible shell into the ground;

FIG. 14 shows a flow diagram of an example of a method of using the open-end extensible shell to form a support pier;

FIG. 15A and FIG. 15B show certain process steps of using the open-end extensible shell to form a pier; and

FIG. 16 is a graph showing results of load tests of a support pier constructed using an embodiment as shown in FIG. 10A, FIG. 10B and/or FIG. 11A, FIG. 11B, FIG. 11C.

DETAILED DESCRIPTION OF THE INVENTION

The present invention is directed to an extensible shell and related methods for constructing a support “shell pier” in ground. Particularly, an extensible shell in accordance with embodiments of the present invention can have an interior into which granular construction material can be loaded and compacted. The shell can be positioned in a cavity formed in the ground (the cavity being formed through a variety of methods as described in more detail below, including driving the shell from grade to form the cavity). After positioning in the ground, granular construction material can be loaded into the interior through an opening of the shell. The granular construction material may be subsequently compacted. The shell can be extensible (or flexible) such that walls of the shell expand when the granular construction material is compacted in the interior of the shell. Therefore, since the shell maintains the compacted granular construction material in a contained manner (i.e., the material cannot expand laterally beyond the shell walls into the in-situ soil) the ground surrounding the shell is reinforced and improved for supporting shallow foundations and other structures. The present invention can be advantageous, for example, because it allows for much higher load carrying capacity due to its ability to limit the granular construction material from bulging laterally outward during loading. The shell is typically made of relatively permanent, substantially non-corrosive and/or non-degradable material such that the lateral bulging of the material is limited for the life of the pier.

FIGS. 1A-1E illustrate different views of an extensible shell 100 in accordance with embodiments of the present invention. FIG. 1A depicts a perspective view of the extensible shell 100, which includes an enclosed end 102. The surface of the enclosed end 102 can define a substantially flat, blunt bottom surface 104, which can be hexagonal in shape. In the alternative, the enclosed end 102 may have any other suitable shape or size. Further, the bottom of the shell may be open, or may be blunt as in the case of a cylindrical shell, may be pointed as the bottom of a conical shell, or may be truncated to form a blunt shape at the bottom of conical or articulated section such as, for example, a frustum, or

frustoconical configuration. It is therefore understood, for the purposes of this disclosure, that the term conical includes frustoconical configurations. The length of the shell may range from about 0.5 m to about 20 m long; such as from about 1 m to about 10 m long. The surfaces of the shell (inside and/or outside) may be smooth or contain a varying degree of roughness for interaction with surrounding surfaces.

Opposing the enclosed end **102** is another end, open end **106**, which defines an opening **108** for receiving granular construction material into an interior (not shown in FIG. 1A) defined by the shell **100**. As will be described in further detail herein below, the open end **106** is positioned substantial vertical to and above from the enclosed end **102** during construction of the pier.

FIGS. 1B, 1C, 1D, and 1E depict a top view, bottom view, a side view, and a cross-sectional side view of the extensible shell **100**, respectively. As shown in FIG. 1B, the extensible shell **100** defines a substantially hollow interior **110** extending between the open end **106** (with opening **108**) and the enclosed end **102**.

FIG. 1C shows that a cross-section of the open end **106** may be sized larger than the bottom surface **104** of the enclosed end **102**. FIG. 1D shows section line A-A arrows indicating the direction of the cross-sectional side view of the extensible shell **100** depicted in FIG. 1E.

The shape of the exterior of the shell **100** may be articulated to form a plurality of panels that form a hexagonal shape in cross-section as viewed from the top or bottom of the shell. Alternatively, the shape may be octagonal, cylindrical, conical, or any other suitable shape.

The extensible shell **100** is often shaped to taper downward from the open end **106** to the enclosed end **102**. In one embodiment, the shell **100** tapers at a 2 degree angle, although the shell may taper at any other suitable angle.

The extensible shell **100** may be made of plastic, aluminum, or any metallic or non-metallic material of suitable extensibility, and preferably substantially non-corrosive and/or non-degradable material. The shell **100** may be relatively thin-walled. The thickness of the wall of the shell **100** may range, for example, from about 0.5 mm to about 100 mm. The example shell **100** of FIG. 1B has a thickness of about 0.25 inches (approximately 6.35 mm), although the shell may have any other suitable thickness. This thickness distance is the distance that uniformly separates the interior **110** and the exterior of the shell. The material of the shell and its thickness may be configured such that the shell has suitable integrity to hold construction material in its interior **110** and to expand laterally at least some distance when the construction material is compacted in the interior **110**.

FIGS. 2A-2C illustrate steps in an exemplary method of constructing a pier in ground using an extensible shell **100** in accordance with an embodiment of the present invention. In this example, side partial cross-section views illustrate the use of the extensible shell **100** for constructing a pier **200** in the ground (see FIG. 2C) in accordance with an embodiment of the present invention. Other methods are described with reference to FIGS. 3A-3D and the Examples below. The method of FIGS. 2A-2C includes forming a pre-formed elongate vertical cavity **202** or hole in a ground surface **204**, as shown in FIG. 2A. The ground may be comprised of primarily soft cohesive soil such as soft clay and silt, or also loose sand, fill materials, or the like. The cavity **202** may be formed with a suitable drilling device having, for example, a drill head or auger for forming a cavity or hole, or may be formed by other methods for forming a cavity such as by inserting and removing a driving mandrel to the desired

pre-formed cavity depth. In some embodiments, the cavity may not be formed at all prior to shell insertion, such as described below with reference to FIGS. 3A-3D.

After the partial cavity **202** has been formed, the extensible shell **100** may be positioned within the cavity **202**, as shown in FIG. 2B, for ultimate driving to the desired depth. Particularly, an extractable mandrel **206** may be used for driving the extensible shell **100** into the cavity **202** and ground **204**. A tamper head **208** of the mandrel **206** may be positioned against a bottom surface **210** of the interior **110** and used to drive the shell **100** to the desired penetration depth, as shown in FIG. 2C. The cavity **202** is at that point formed of a size and dimension such that the exterior surface of the extensible shell **100** fits tightly against the walls of the cavity **202**.

After the extensible shell **100** has been driven into (while forming) the fully enlarged cavity **202**, the mandrel **206** is removed, leaving behind the shell **100** in the cavity **202** and with the interior **110** being empty. The shell **100** may then be filled with a granular construction material **212**, such as sand, aggregate, admixture-stabilized sand or aggregate, recycled materials, crushed glass, or other suitable materials as shown in FIG. 2C. The granular construction material **212** may be compacted within the shell using the mandrel **206**. The compaction increases the strength and stiffness of the internal granular construction material **212** and pushes the granular construction material **212** outward against the walls of the shell **100**, which pre-strains the shell **100** and increases the coupling of the shell **100** with the in-situ soil. Significant increases in the load carrying capacity of the pier **200** can be achieved as a result of the restraint offered by the shell **100**.

FIGS. 3A-3D illustrate steps in another exemplary method of constructing a pier in ground using an extensible shell in accordance with an embodiment of the present invention. Referring to FIG. 3A, an aggregate construction material **300** (e.g., sand) is placed in the interior **110** of the shell **100** to a predetermined level above the bottom surface **210** of the shell **100**. Next, the tamper head **208** of the extractable mandrel **206** is fitted to the interior **110** of the extensible shell **100**, and against the top of the aggregate construction material **300**. The mandrel **206** may then be moved towards the ground **204** in a direction indicated by arrow **302** for driving the shell **100** into the ground **204**. Driving may be facilitated using a small pre-formed cavity (e.g., the cavity **202** shown in FIG. 2A), or not, depending on site conditions.

Referring to FIG. 3B, the mandrel **206** is shown driving the shell **100** into the ground **204** in the direction **302** such that the shell **100** is at a predetermined depth below grade. Next, the mandrel **206** may be removed. At FIG. 3C, the shell **100** is substantially filled with additional aggregate construction material **304** (e.g., sand) through opening **108**, and the mandrel **206** is positioned as shown. Next, vertical compaction force and/or vibratory energy is applied to the mandrel **206** for compacting the materials **300** and **304**. The shell **100** may be driven by this force to a further depth below grade. The addition of construction material **304** and subsequent compaction can be repeated several times until the final pier is constructed. Alternatively, the shell may be "topped off" with additional construction material after only one compaction cycle.

In an embodiment of the present invention, a second mandrel **212** may be used to compact the upper portion of the material **304** in the direction **302**, as shown in FIG. 3D. The second mandrel **212** may have a larger cross-sectional

area than the primary mandrel **206** to provide increased confinement during compaction.

In an embodiment of the present invention, the shell **100** may define apertures **218** that extend between the interior **110** and an exterior of the shell **100** to the in-situ soil (see FIGS. 1A and 2C). The apertures **218** may provide for drainage of excess pore water pressure that may exist in the in-situ soil to drain into the interior **110** of the shell **100**. Increases in pore water pressure typically decreases the strength of the soil and is one of the reasons that prior art piers are limited in their load carrying capacity in saturated cohesive soil such as clay, silt, or the like. The apertures **218** envisioned herein allow the excess pore water pressure in the soil to dissipate into the pier **200** after insertion. This allows the in-situ soil to quickly gain strength with time, a phenomena not enjoyed by concrete, steel piles, or grout elements (i.e., “hardened” elements). The drainage of excess pore water pressures allows additional settlement of the soil that may occur as a result of pore water pressure dissipation prior to the application of foundation loads.

Other embodiments may not define apertures, or may provide one or more apertures **218** on only one side of the shell **100**. Alternatively, the apertures **218** may be defined in the shell **100** such that they are positioned along a portion of the length of the shell **100**, are positioned along the full length of the shell **100**, or may be positioned asymmetrically in various configurations. The sizes and placements of the apertures **218** can vary according to the size of the shell **100**, the conditions of the ground (e.g., where higher water pressure is known to exist), and other relevant factors. The apertures **218** may range in size from about 0.5 mm to about 50 mm; such as from about 1 mm to about 25 mm. In another embodiment, the top of the shell **100** may be enclosed and connected to vacuum pressure to further increase and accelerate drainage of excess water pressure in the surrounding soil through the apertures **218**.

The mandrel **206** may be constructed of sufficient strength, stiffness, and geometry to adequately support the shell **100** during driving and to be able to be retracted from the shell **100** after driving. In one embodiment, the shape of the exterior of mandrel **206** is substantially similar to the shape of the interior **110** defined by the shell **100**. In another embodiment, the mandrel **206** is comprised primarily of steel. Other materials are also envisioned including, but not limited to, aluminum, hard composite materials, and the like.

The mandrel **206** may be driven by a piling machine or other suitable equipment and technique that may apply static crowd pressure, hammering, or vibration sufficient to drive the mandrel **206** and extensible shell **100** into the surface of ground **204**. In one embodiment, the machine may be comprised of an articulating, diesel, pile-driving hammer that drives the mandrel **206** using high energy impact forces. The hammer may be mounted on leads suspended from a crane. In another embodiment, the hammer may be a sheet pile vibrator mounted on a rig capable of supplying a downward static force. In another embodiment, the shell **100** may be placed in a pre-formed cavity **200** and constructed without the use of an extractable mandrel. Standard methods of driving mandrels into the ground are known in the art and therefore, can be used for driving.

The following Examples illustrate further aspects of the invention.

Example I

As an example, piers were constructed using extensible shells in accordance with embodiments of the present inven-

tion at a test site in Iowa. Load tests were conducted on the piers using a conventional process. The extensible shells used in the tests and the methods of their use consisted essentially of that described above and shown in the attached Figures. In this test, extensible shells formed from LEXAN® polycarbonate plastic were installed at a test site characterized by soft clay soil. This testing was designed to compare the load versus deflection characteristics of an extensible shell in accordance with the present invention to aggregate piers constructed using a driven tapered pipe. Two comparison aggregate piers (of fine and coarse aggregate) were constructed to a depth of 12 feet below the ground surface.

In this test, the extensible shell was formed by bending sheets of the plastic to form a tapered shape having a hexagonal cross-section and that tapered downward from an outside diameter of 24 inches (610 mm) at the top of the shell to a diameter of 18 inches (460 mm) at the bottom of the shell. A panel of the shells overlapped, and this portion was both glued and bolted together. The length of the extensible shell was 9.5 feet (2.9 m). In this embodiment, apertures were formed in the extensible shell by perforating the sides of the shell with 3 mm to 7 mm diameter “weep” holes spaced apart from each another. The bottom portion of the shell was capped with a steel shoe to facilitate driving. LEXAN® polycarbonate plastic has a tensile strength of approximately 16 MPa (2300 psi) at 11 percent elongation and a Young’s modulus of 540 MPa (78,000 psi). The extractable mandrel used in this test was attached to a high frequency hammer, which is often associated with driving sheet piles. The hammer is capable of providing both downward force and vibratory energy for driving the shell into the ground and for compacting aggregate construction material in the shell.

In this example, the extensible shell was driven into the ground without pre-drilling of the cavity or hole. Particularly, in this test, the two shells were installed by orientating each shell in a vertical direction, placing approximately 4 feet (1.2 m) of sand at the base of the shell, and then driving the shell into the ground surface with an extractable mandrel with exterior dimensions similar to those of the interior of the shell. The shell was driven to a depth of approximately 8.5 feet (2.6 m) below grade. The mandrel was removed and the shells were filled with sand. The extractable mandrel was then re-lowered within the shells and vertical compaction force in combination with vibratory energy was applied to both compact the sand to drive the shell to a depth of 9 feet (2.7 m) below grade. The mandrel was then extracted and the upper portion of the shell was then filled with crushed stone to a depth of 0.5 ft (0.2 m) below grade. A concrete cap was then poured above the crushed stone fill to facilitate load testing.

Radial cracks were observed to extend outward from the edge of the shell pier. These cracks form drainage galleries that are the result of high radial stresses and low tangential stresses created in the ground during pier installation. Drainage was afforded by the perforations in the shell and allowed soil water to drain into the sand and aggregate filled piers.

The shell piers were load tested using a hydraulic jack pushing against a test frame. FIG. 4 is a graph showing results of the load test compared with aggregate piers constructed using a similarly shaped mandrel. As shown in FIG. 4, at a top of pier deflection of one inch, the piers constructed without shells supported a load of 15,000 pounds to 20,000 pounds (67 kN to 89 kN). The shell piers constructed in this embodiment of the invention supported a load of 310 kN to 360 kN (70,000 to 80,000 pounds) at a top

of pier deflection of one inch. The load carrying capacity of the shell piers constructed in accordance with the present invention provided a 3.5 to 5.3 fold improvement when compared to aggregate piers constructed without extensible shells.

Example II

In other testing, extensible shells were formed from high-density polyethylene polymer ("HDPE") and installed at the test site as described in Example I. This testing program was designed to compare the load versus deflection characteristics of this embodiment of the present invention to aggregate piers constructed using a driven tapered pipe as described in Example I. A total of six shell piers were installed as part of this example.

In this test, the extensible shell was formed by a rotomolding process. The shells defined a tapered shape having a hexagonal cross-section and that tapered downward from an outside diameter of 585 mm (23 inches) at the top of the shell to a diameter of 460 mm (18 inches) at the bottom of the shell. The bottom of the extensible shell was integrally constructed as part of the shell walls as a result of the rotomolding process. The mandrel in this embodiment was attached to the same hammer as described in Example I.

The installation process in this Example was somewhat different from that in Example I and included pre-drilling a 30 inch (0.76 m) diameter cavity to a depth of 2 feet (0.61 m) to 3 feet (0.9 m) below the ground surface (rather than driving the shell initially from top grade). The shell was then placed vertically in the pre-drilled cavity. The extractable mandrel was then inserted into the shell, and the shell was driven to a depth 11 feet (3.4 m) to 12 feet (3.7 m) below grade. The extensible shell was then filled with aggregate construction material and compacted in four lifts; with each lift about 7.4 cubic feet (0.2 cubic meters) in volume. The aggregate consisted of sand in five of the piers and consisted of crushed stone in one of the piers. Each lift was compacted with the downward pressure and vibratory energy of the extractable mandrel.

After placement and compaction of sand within the extensible shells, the top of the shells were situated at about 2 feet (0.61 m) to 3 feet (0.9 m) below the ground surface. Crushed stone was then placed and compacted above the extensible shell to a depth of 1 foot (0.3 m) below the ground surface. A concrete cap was then poured above the crushed stone fill to facilitate load testing.

The shell piers were load tested using a hydraulic jack pushing against a test frame. FIG. 5 is a graph showing results of the load test compared with the aggregate piers described in Example I. As shown in FIG. 5, at a top of pier deflection of one inch, the piers constructed without shells supported a load of 15,000 pounds to 20,000 pounds (67 kN to 89 kN). The shell piers constructed in this embodiment of the invention supported loads ranging from 62,000 pounds (275 kN) to 71,000 pounds (315 kN) at the top of pier deflections of one inch. The load carrying capacity of the shell piers constructed in accordance with this embodiment of the present invention provided a 3.1 to 4.7 fold improvement when compared to aggregate piers constructed without extensible shells.

Example III

In another test, an extensible shell of the same embodiment described in Example II was installed at the test site as described in Example I. This testing program was designed

to compare the load versus deflection characteristics of this embodiment of the invention to aggregate piers constructed using a driven tapered pipe as described in Example I. The mandrel, hammer, and extensible shell used for testing were the same as used in Example II.

In this embodiment of the present invention, the installation process included pre-drilling a 30 inch (0.76 m) diameter cavity to a depth of 3 feet (0.9 m) below the ground surface. The extractable mandrel was then inserted into the pre-drilled cavity, to create a cavity with a total depth of 5 feet (1.5 m) below the ground surface. This cavity was then backfilled to the ground surface with sand. The extensible shell was then driven vertically through the sand filled cavity with the extractable mandrel to a depth of 9 feet (2.7 m) below the ground surface, so that the top of the shell was situated 6 inches above the ground surface. The extensible shell was then filled with sand in four lifts, with each lift about 7.4 cubic feet (0.2 cubic meters) in volume. Each lift was compacted with the downward pressure and vibratory energy of the mandrel. A concrete cap encompassing the top of the shell was then cast over the shell to facilitate load testing.

The shell pier was load tested using a hydraulic jack pushing against a test frame. FIG. 6 is a graph showing results of the load test compared with the aggregate piers described in Example I. As shown in FIG. 6, at a top of pier deflection of one inch, the piers constructed without shells supported a load of 15,000 pounds to 20,000 pounds (67 kN to 89 kN). The pier constructed in this embodiment of the present invention supported a load of 57,500 pounds (255 kN) with a top of pier deflection of one inch. The load carrying capacity of the shell pier constructed in accordance with this embodiment of the present invention provided a 2.9 to 3.8 fold improvement when compared to aggregate piers constructed without extensible shells.

Example IV

In yet another test, an embodiment of the present invention was installed at a project site characterized by 3 feet (0.9 m) of loose sand soil over 7 feet (2.1 m) of soft clay soil over dense sand soil. The embodiment of the present invention at the project site was used to support structural loads, such as those associated with building foundations and heavily loaded floor slabs. The mandrel, hammer, and extensible shell used for testing were the same as used in Examples II and III.

In this embodiment of the present invention, the installation process included pre-drilling a 30 inch (0.76 m) diameter pre-drill to a depth of 3 feet (0.9 m) below the ground surface. Approximately 7.4 cubic feet (0.2 cubic meters) of sand was then placed in the pre-drilled cavity. This resulted in the pre-drilled cavity being about half-full.

The extensible shell was then placed vertically in the partially backfilled pre-drilled cavity. The extractable mandrel was then inserted into the shell, and the shell was driven to a depth 12.5 feet (3.8 m) below grade. The extensible shell was then filled with sand in four lifts; with each lift about 7.4 cubic feet (0.2 cubic meters) in volume. Each lift was compacted with the downward pressure and vibratory energy of the mandrel.

After placement and compaction of sand within the extensible shell, a lift of crushed stone about 4.9 cubic feet (0.14 cubic meters) in volume was placed and compacted within the extensible shell. Crushed stone was then placed and compacted above the extensible shell until the crushed stone backfill was level with the ground surface.

At one shell location, a 30 inch (0.76 m) diameter concrete cap was placed over the shell to facilitate load testing. At a second shell location, a 6 foot (1.8 m) wide by 6 foot (1.8 m) wide concrete cap was placed over the shell to facilitate loading and to measure the load deflection characteristics of the composite of native matrix soil and extensible shell (to simulate a floor slab).

The shell piers were load tested using a hydraulic jack pushing against a test frame, with the results of the load testing being shown in FIG. 7. The shell pier tested with the 30 inch diameter concrete cap supported a load of 35,500 pounds (158 kN) at a deflection of 0.4 inches (10 mm). The shell pier tested with a 6 foot wide by 6 foot wide concrete cap supported a load of 104,700 pounds (467 kN) at a deflection of 0.4 inches (10 mm).

Slotted Shell Embodiment

With reference to FIG. 8, an alternative embodiment of the present invention is shown and which includes an extensible shell **800** with one or more slits or slots **812** that extend between an interior of the shell to an exterior of the shell. The slots **812** may be placed over the entire length of the shell **800** or only partially located along the length and have varying spacing, such as, for example, slots being spaced every 6 inches (152 mm) starting generally 1.5 foot (0.46 m) from the top and bottom. The slots **812** may be of varying widths, such as, for example, $\frac{1}{4}$ inch (6.35 mm) to $\frac{3}{8}$ inch (9.53 mm) wide. The slots **812** typically run generally transverse to a centerline along the length of the shell and may form a minor or major part of the circumference of the shell **800**. In one embodiment, such as shown in FIG. 8, the slots **812** are discontinuous around the circumference leaving three spines **814** to maintain portions of continuous material connectivity along the length of the shell **800**. The shell **800** of this embodiment may be of any suitable size or shape as described above with reference to shell **100**.

As an example, a slotted extensible shell of this embodiment was installed at a test site in Iowa to compare the load versus deflection characteristics of this embodiment of the extensible shell to aggregate piers constructed using a driven tapered pipe. The test site was characterized by soft clay soil and the two comparison aggregate piers (of fine and coarse aggregate) were constructed to a depth of 12 feet below the ground surface.

For this test of the extensible shell, the shell was formed from High Density Polyethylene polymer and was formed by the rotomolding process. The shell formed a tapered shape that was hexagonal in cross section and tapered downward from an outside diameter of 23 inches (585 mm) at the top of the shell to a diameter of 18 inches (460 mm) at the bottom of the shell. The bottom of this embodiment of the extensible shell was integrally constructed as part of the shell walls as a result of the rotomolding process. In this embodiment of the invention (similar to that shown in FIG. 8), $\frac{1}{4}$ inch (6.35 mm) wide slots were cut in a circumferential orientation around the extensible shell. The extensible shell was left as a single continuous piece, by not removing material from three of the six corners or spines. The extractable mandrel used in this test was attached to a high frequency hammer, which is often associated with driving sheet piles. The hammer is capable of providing both downward force and vibratory energy for driving the shell into the ground and for compacting aggregate construction material in the shell.

In this example, the installation process included a 30 inch (0.76 m) diameter pre-drill to a depth of 1.5 feet (0.46 m)

below the ground surface. The shell was then placed vertically in the pre-drilled hole and then the shell was driven with an extractable mandrel with exterior dimensions similar to those of the interior of the shell. The shell was driven to a depth of 11 feet (3.4 m) below grade. The mandrel was removed and the extensible shell was then filled with aggregate in four lifts; with each lift about 7.4 cubic feet (0.2 cubic meters) in volume. Each lift was compacted with the downward pressure and vibratory energy of the extractable mandrel.

After placement and compaction of aggregate within the extensible shell, the top of the shell was situated at about 1.5 feet (0.46 m) below the ground surface. The aggregate backfill was then leveled with the top of the shell, and a concrete cap was then poured above the shell to facilitate load testing.

The slotted shell pier was load tested using a hydraulic jack pushing against a test frame. FIG. 9 is a graph showing results of the load test compared with the aggregate piers described above. As shown in FIG. 9, at a top of pier deflection of one inch, the piers constructed without slotted shells supported a load of 15,000 pounds to 20,000 pounds (67 kN to 89 kN). The pier constructed in this embodiment of the invention supported a load of 77,500 pounds (345 kN) at a top of pier deflection of one inch. The load carrying capacity of the pier constructed in accordance with this embodiment of the invention provided a 3.9 to 5.2 fold improvement when compared to aggregate piers constructed without extensible shells.

Open-End Embodiment

With reference to FIGS. 10A through 15B, an alternative embodiment of the present invention is shown and which includes an open-end extensible shell that can be used to form piers. Namely, FIG. 10A shows a perspective view of an example of an open-end extensible shell **1000**. FIG. 10B shows a cross-sectional view of open-end extensible shell **1000** taken along line A-A for FIG. 10A. In this example, open-end extensible shell **1000** is a hollow tubular member that has a first open end **1010** and a second open end **1012**. Open-end extensible shell **1000** can be used in any orientation with respect to driving into the ground. However, for illustration purposes, first open end **1010** is hereafter referred to as advancing open end **1010**, wherein advancing open end **1010** means the bottom end of open-end extensible shell **1000** that is advanced into the ground first. Further, second open end **1012** is hereafter referred to as trailing open end **1012**, wherein trailing open end **1012** means the top end of open-end extensible shell **1000** that is mated to driving equipment, such as a mandrel.

Open-end extensible shell **1000** can be any length and any width or diameter. Without limitation, the length of open-end extensible shell **1000** can be from about 3.05 m (5 ft) to about 6.1 m (20 ft) in one example, or can be about 3.05 m (10 ft) in another example. Without limitation, the width or diameter of open-end extensible shell **1000** can be from about 61 cm (24 in) to about 46 cm (18 in) in one example, or can be about 51.8 cm (20.4 in) in another example. In one example, open-end extensible shell **1000** can be formed of plastic, such as high-density polyethylene polymer (HDPE) plastic. In another example, open-end extensible shell **1000** can be formed of metal, such as steel or aluminum.

Open-end extensible shell **1000** is not limited to a straight tubular shape. For example, FIGS. 11A, 11B, and 11C illustrate various views of an example of an open-end extensible shell **100** that has a hexagon-shaped cross-section

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and a tapered tip; namely, advancing open end 1010 is tapered. Namely, FIGS. 11A and 11B show perspective views of the advancing open end 1010 portion of open-end extensible shell 100, which is hexagonal and includes a taper 1020. FIG. 11C shows a cross-sectional view of open-end extensible shell 1000 taken along line B-B for FIG. 11B. In one example, the width or diameter of open-end extensible shell 100 is tapered from about 51.8 cm (20.4 in) to about 46 cm (18.1 in).

FIGS. 12A and 12B show an example of a process of installing open-end extensible shell 1000 into the ground (e.g., ground 1205). In this example, a closed pipe mandrel 1210 that has a shoulder collar 1215 is used to drive open-end extensible shell 1000 into ground 1205. Closed pipe mandrel 1210 is inserted into open-end extensible shell 1000 until shoulder collar 1215 contacts trailing open end 1012 of open-end extensible shell 1000. In this way, driving force is transferred from closed pipe mandrel 1210 to open-end extensible shell 1000. In FIGS. 12A and 12B, the advancing end of closed pipe mandrel 1210 extends beyond advancing open end 1010 of open-end extensible shell 1000. In one example, the end of closed pipe mandrel 1210 extends about 1.5 m (5 ft) beyond advancing open end 1010 of open-end extensible shell 1000.

However, the position of shoulder collar 1215 can be adjustable along the length of closed pipe mandrel 1210. Namely, shoulder collar 1215 can be adjustable such that a range of depths and relative positions of open-end extensible shell 1000 and closed pipe mandrel 1210 can be achieved without the need to change mandrels. For example, FIG. 13 shows the position of shoulder collar 1215 set such that the advancing end of closed pipe mandrel 1210 substantially aligns with advancing open end 1010 of open-end extensible shell 1000.

FIG. 14 shows a flow diagram of an example of a method 1400 of using open-end extensible shell 1000 to form a support pier. Method 1400 may include, but is not limited to, the following steps.

At a step 1410, open-end extensible shell 1000 is driven into the ground using a mandrel. For example and referring again to FIGS. 12A and 12B, open-end extensible shell 1000 is driven into ground 1205 using closed pipe mandrel 1210.

At a step 1415, the mandrel (e.g., closed pipe mandrel 1210) is withdrawn from open-end extensible shell 1000, leaving open-end extensible shell 1000 in the ground. For example, FIG. 15A shows open-end extensible shell 1000 in ground 1205 after closed pipe mandrel 1210 is withdrawn, creating a shell cavity 1220. Namely, shell cavity 1220 is a portion of ground 1205 that is void of material.

At a step 1420, shell cavity 1220 is backfilled with sand, aggregate, cementitious grout, and/or any other material. For example, FIG. 15B shows shell cavity 1220 of open-end extensible shell 1000 backfilled with a volume of material 1225.

At a step 1425, the mandrel (e.g., closed pipe mandrel 1210) is reinserted into open-end extensible shell 1000. Then, material 1225 is packed to below advancing open end 1010 of open-end extensible shell 1000. For example, FIG. 15B shows a "bulb" of material 1225 is formed in ground 1205 below advancing open end 1010 of open-end extensible shell 1000.

At a step 1430, the mandrel (e.g., closed pipe mandrel 1210) is withdrawn from open-end extensible shell 1000, again as shown in FIG. 15A.

At a step 1435, the remaining portion of shell cavity 1220 is backfilled with material 1225 (e.g., sand, aggregate, cementitious grout, and/or any other material).

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At a step 1440, the mandrel (e.g., closed pipe mandrel 1210) is reinserted into open-end extensible shell 1000. Then, material 1225 is packed into shell cavity 1220 of open-end extensible shell 1000.

At a step 1445, the mandrel (e.g., closed pipe mandrel 1210) is withdrawn from open-end extensible shell 1000, again as shown in FIG. 15A.

At a decision step 1450, it is determined whether the construction of the support pier is complete. If the construction of the support pier is complete, then method 1400 ends. However, if the construction of the support pier is not complete, then method 1400 returns to 1435.

A benefit of using open-end extensible shell 1000 and method 1400 is that it provides increased stiffness for the shell support layer and increased overall length of the extensible shell system in the upper zone (open-end extensible shell 1000 plus "bulb" depth).

Example V

As an example, support piers were constructed using extensible shells in accordance with embodiments of the present invention at a test site in Iowa. Load tests were conducted on the piers using a conventional process. The extensible shells used in the tests and the methods of their use consisted essentially of that described above and shown in FIGS. 10A through 15B. In this test, extensible shells formed of high-density polyethylene polymer (HDPE) plastic were installed at a test site characterized by soft clay soil. This testing was designed to compare the load versus deflection characteristics of an extensible shell in accordance with the present invention to aggregate piers constructed with a driven tapered pipe. Two comparison aggregate piers were constructed to a depth of 12 feet below the ground surface.

In this test, the extensible shell was formed by a rotomolding process. The shells defined a tapered shape having a hexagonal cross-section (e.g., as shown in FIGS. 11A, 11B, 11C) and that tapered downward from an outside diameter of 518 mm (20.4 inches) at the top of the shell to a diameter of 460 mm (18.1 inches) at the bottom of the shell. In this embodiment of the invention the extensible shell has a total length of 3.05 m (10 feet), and both the top and the bottom ends of the shell are open such that and extractable tapered mandrel commonly used for constructing aggregate piers could fully pass through the extensible shell.

The extractable mandrel used in this test was attached to a high frequency hammer, which is often associated with driving sheet piles. The hammer is capable of providing both downward force and vibratory energy for driving the shell into the ground and for compacting aggregate construction material in the shell. The "open bottom" extensible shell pier and the aggregate pier were constructed with a similar mandrel and high frequency hammer.

In this example, a 61 cm (24 in) diameter and 61 cm (24 in) deep pre-drill hole was formed at the ground surface prior to driving the extensible shell. The purpose of the pre-drill is to facilitate the placement of a concrete cap for the load test. The extensible shell, and Tapered Mandrel were then driven into the ground such that the tip of the tapered mandrel was at a depth of about 5.2 m (17 ft) below the ground surface, the bottom of the extensible shell was at a depth of about 3.65 m (12 ft) below the ground surface, and the top of the shell was at a depth of about 61 cm (24 in) below the ground surface.

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The tapered mandrel used in this example is hollow such that such that the mandrel can be filled with aggregate, and allowed to flow out the bottom of the mandrel. An aggregate pier is constructed with this mandrel by raising and lowering the mandrel pre-determined distances to construct the aggregate pier. In this example, an aggregate pier was constructed below and within the extensible shell using a similar process.

The open bottom extensible shell piers were load tested using a hydraulic jack pushing against a test frame. FIG. 16 is a graph showing results of the load test compared with aggregate piers constructed using an embodiment as shown in FIGS. 10A, 10B and/or FIGS. 11A, 11B, 11C. As shown in FIG. 16, at a top of pier deflection of one inch, the piers constructed without shells supported a load of 67 kN to 89 kN (15,000 pounds to 20,000 pounds). The piers constructed in this embodiment of the invention supported a load of 188 kN (42,300 pounds) at a top of pier deflection of one inch. The load carrying capacity of the piers constructed in accordance with the present invention provided a 2.1 to 2.8 fold improvement when compared to aggregate piers constructed without extensible shells.

The foregoing detailed description of embodiments refers to the accompanying drawings, which illustrate specific embodiments of the invention. Other embodiments having different structures and operations do not depart from the scope of the invention. The term "the invention" or the like is used with reference to certain specific examples of the many alternative aspects or embodiments of the applicant's invention set forth in this specification, and neither its use nor its absence is intended to limit the scope of the applicant's invention or the scope of the claims. Moreover, although the term "step" may be used herein to connote different aspects of methods employed, the term should not be interpreted as implying any particular order among or between various steps herein disclosed unless and except when the order of individual steps is explicitly described. This specification is divided into sections for the convenience of the reader only. Headings should not be construed as limiting of the scope of the invention. It will be understood that various details of the invention may be changed without departing from the scope of the invention. Furthermore, the foregoing description is for the purpose of illustration only, and not for the purpose of limitation.

What is claimed:

1. A method for constructing a support pier in ground, the method comprising:

- (a) positioning an extensible shell into ground, the shell defining an interior for holding granular construction material and defining a first opening at a first end for receiving the granular construction material into the interior and defining a second opening at an opposing second end, wherein the shell is flexible such that the shell expands when granular construction material is compacted in the interior of the shell;
- (b) filling at least a portion of the interior of the shell with the granular construction material; and
- (c) compacting the granular construction material in the interior of the shell to form a support pier.

2. The method of claim 1, wherein the shell is shaped to taper downward from the first end to the opposing second end of the shell.

3. The method of claim 2, wherein a cross-section of the shell forms one of a substantially hexagonal shape and a substantially octagonal shape along a length of the shell extending between the first and second ends.

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4. The method of claim 2, wherein a cross-section of the first end is sized larger than a cross-section of the second end.

5. The method of claim 1, wherein the shell is comprised of plastic.

6. The method of claim 1, wherein the shell defines a plurality of apertures extending between the interior of the shell to an exterior of the shell.

7. The method of claim 6, further comprising applying vacuum pressure through the shell.

8. The method of claim 1, wherein the shell is substantially cylindrical in shape.

9. The method of claim 1, wherein the shell is substantially conical in shape.

10. The method of claim 1, wherein the positioning in step (a) further comprises partially filling the shell with the granular construction material and driving the shell into the ground subsequent to partially filling the shell.

11. The method of claim 10, wherein driving the extensible shell comprises applying a force to a mandrel for driving the shell into the ground.

12. The method of claim 1, wherein the positioning in step (a) further comprises first forming a cavity in the ground and subsequently driving the extensible shell into the cavity.

13. The method of claim 12, wherein the cavity is at least partially filled with granular construction material after forming and prior to the driving of the extensible shell into the cavity.

14. The method of claim 1, wherein the compacting in step (c) is performed with a primary mandrel.

15. The method of claim 14, further comprising an additional compacting step performed with a second mandrel that has a larger cross-sectional area than the primary mandrel.

16. The method of claim 1, wherein the shell further comprises a plurality of slots extending between an interior of the shell to an exterior of the shell, the slots being generally transverse to a centerline along the length of the shell.

17. The method of claim 16, wherein the slots are discontinuous around a circumference of the shell thereby maintaining portions of continuous material connectivity along the length of the shell.

18. The method of claim 16, wherein the slots have a width in the range of 1/4 inch (6.35 mm) to 3/8 inch (9.53 mm).

19. The method of claim 16, wherein the slots are spaced at a distance of 6 inches (152 mm) from one another.

20. A method for constructing a support pier in ground, the method comprising:

- (a) forming a cavity in the ground;
- (b) partially backfilling the cavity with an aggregate construction material;
- (c) positioning an extensible shell into the cavity, the shell having a first end with a first opening and a second end having a second opening, the shell defining an interior for holding granular construction material and defining an opening for receiving the granular construction material into the interior, wherein the shell is flexible such that the shell expands when granular construction material is compacted in the interior of the shell;
- (d) filling at least a portion of the interior of the shell with the granular construction material; and
- (e) compacting the granular construction material in the interior of the shell to form a support pier.

21. An extensible shell for constructing a support pier in ground, the extensible shell defining an interior for holding

granular construction material and defining a first end having a first opening therethrough for receiving the granular construction material into the interior and an opposing second end having a second opening therethrough, wherein the shell is flexible such that the shell expands laterally outward when granular construction material is compacted in the interior of the shell and further comprising a plurality of slots extending between an interior of the shell to an exterior of the shell, the slots being generally transverse to a centerline along the length of the shell.

22. The extensible shell of claim **21**, wherein the slots are discontinuous around a circumference of the shell thereby maintaining portions of continuous material connectivity along the length of the shell.

23. The extensible shell of claim **21**, wherein the slots have a width in the range of $\frac{1}{4}$ inch (6.35 mm) to $\frac{3}{8}$ inch (9.53 mm).

24. The extensible shell of claim **21**, wherein the slots are spaced at a distance of 6 inches (152 mm) from one another.

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